

A
SEMINAR REPORT
on

**Anaerobic treatment of domestic sewage: established technologies
and perspectives: with Special Emphasis on
UASB**

Submitted in partial fulfillment of the requirement for the degree of

**MASTER OF TECHNOLOGY
IN
ENVIRONMENTAL ENGINEERING
(CIVIL ENGINEERING DEPARTMENT)**



GUIDED BY:

Dr A.B. Gupta

Professor

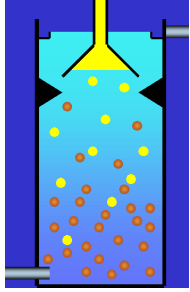
Civil Department

SUBMITTED BY:

Ms. Neetu Dabi

M.Tech IIIrd Sem.

ID-046159



INTRODUCTION

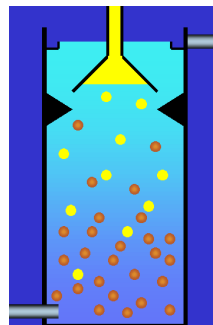
- Sewage is greatest source of aquatic pollution & public health concern in urban areas of developing countries.
- *Domestic sewage* is defined as human excreta, urine, and the associated sludge (collectively known as blackwater), as well as, kitchen wastewater and wastewater generated through bathing (collectively known as greywater).

Several technology in the field of wastewater treatment:

- *Conventional aerobic treatment in ponds
- *Trickling Filters ,RBC ,ASP
- *Anaerobic treatment
- *Combination of Anaerobic And Aerobic Treatment

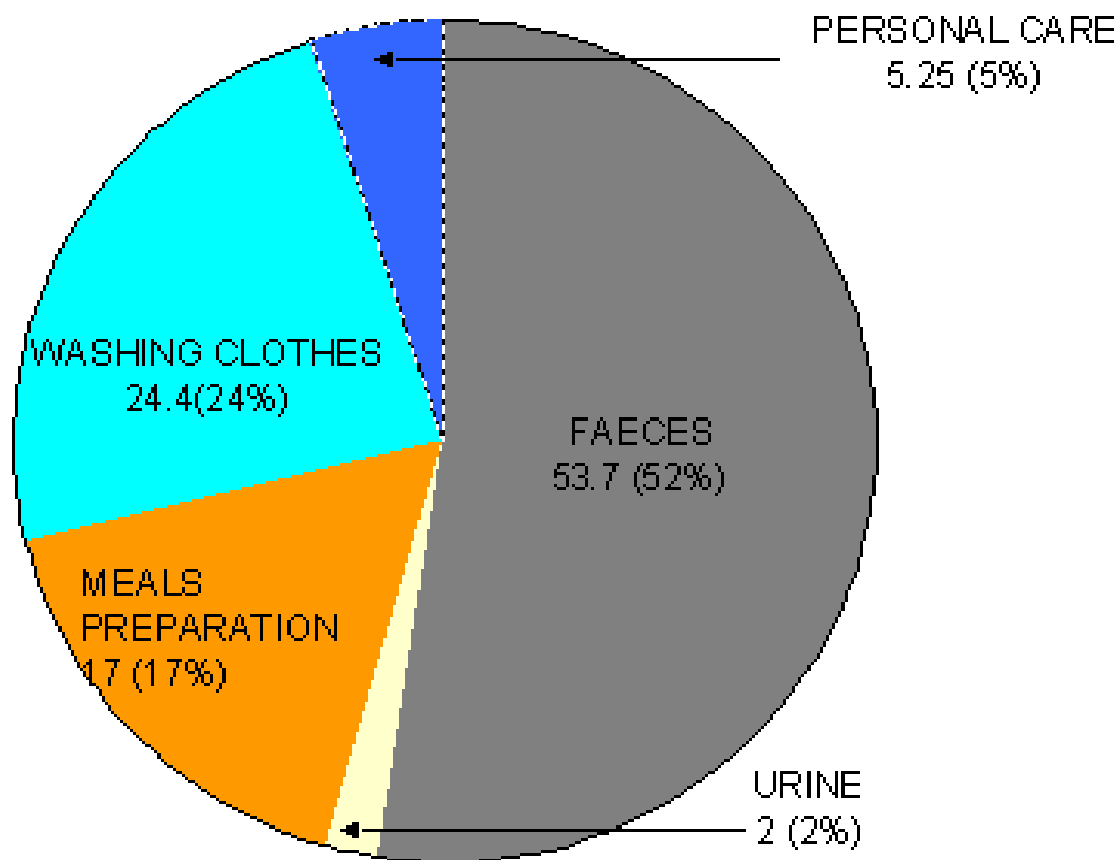
Adequate treatment system have to be :

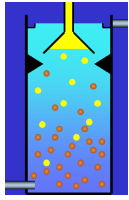
- Simple in design
- Efficient in removing the pollutants
- Energy consumption should be low
- Re use of water for use purpose
- Use of sophisticated equipment must be kept to a minimum.



ORGANIC MATTER IN DOMESTIC WASTEWATER

(gCOD/p/d)





Anaerobic Waste Treatment : An Overview

Historical development:

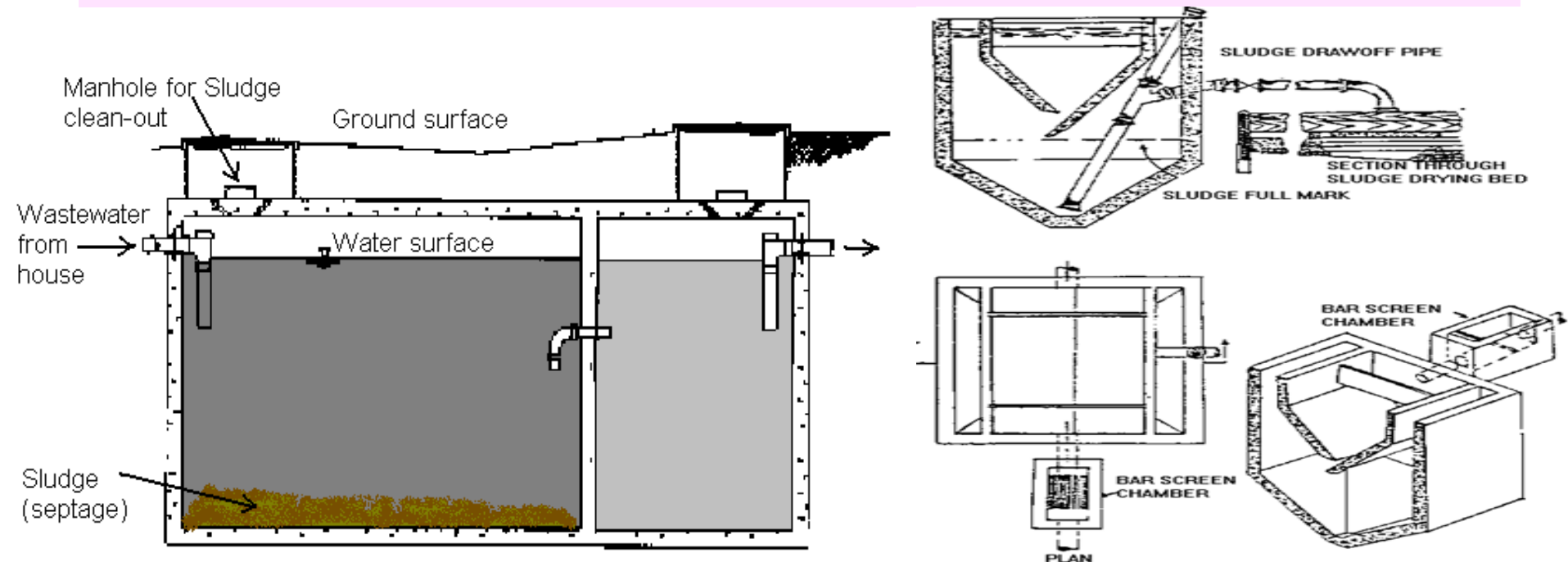
Mainly used for reducing mass of high solids wastes, e.g. human waste (night soil), animal manure, agricultural waste and sludge

Early applications of anaerobic waste treatment include:

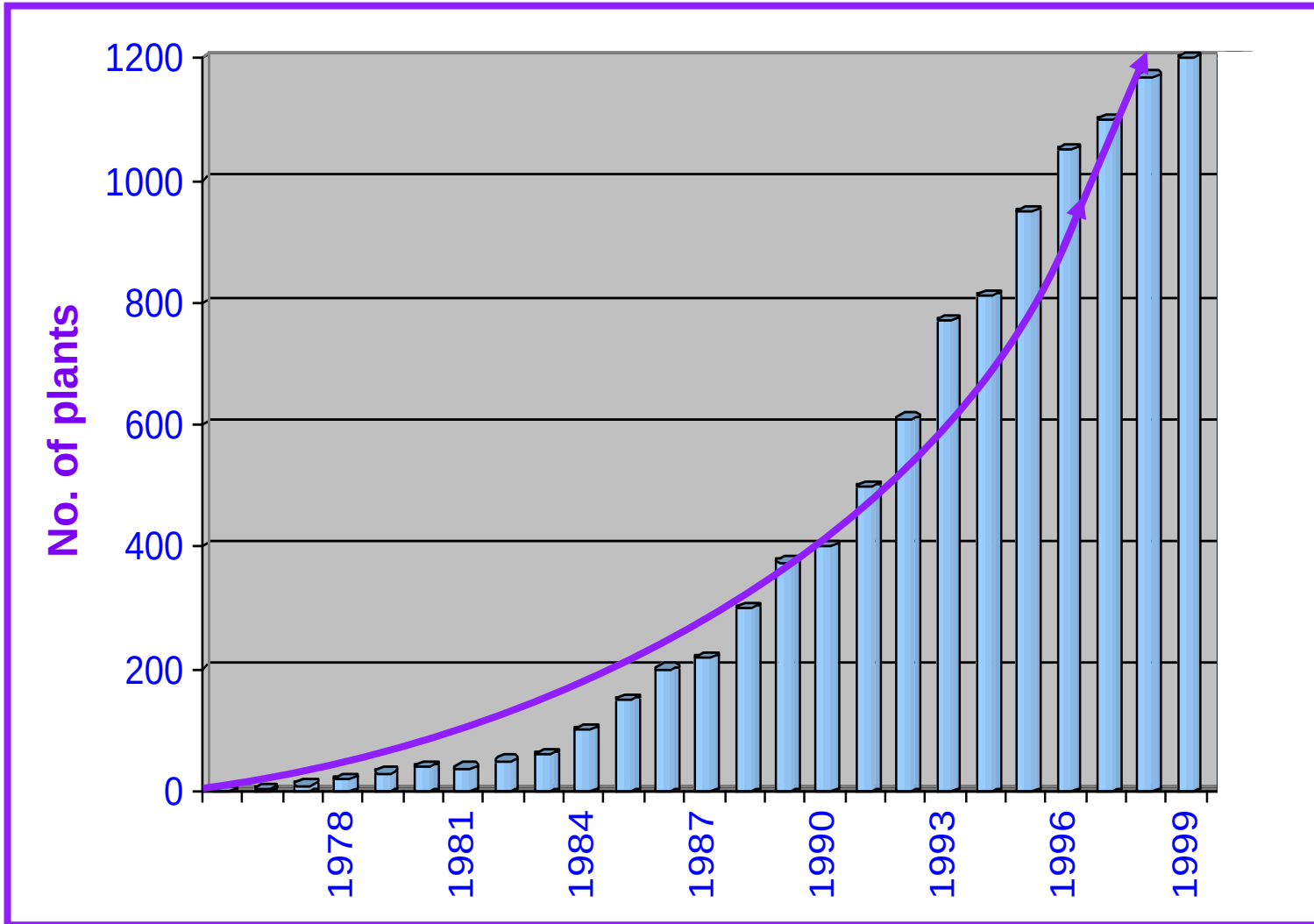
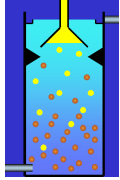
Mouras automatic scavenger - cited in French journal *cosmos* in 1881

Septic tank- developed by Donald Cameron in 1895 (England)

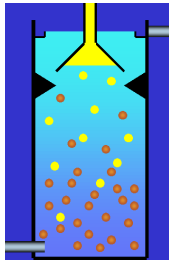
Imhoff tank: developed by Karl Imhoff in 1905 (Germany)



**Aerobic treatment process - considerable progress in short span of time.
Anaerobic technology: energy crisis in 70 and 80's- a renewed interest in anaerobic process**



Anaerobic treatment plants for industrial applications (Source: Frankin, 2001)



Anaerobic Waste Treatment

Definition:

Anaerobic treatment is a biological process carried out in the absence of O_2 for the stabilization of organic materials by conversion to CH_4 and inorganic end-products such as CO_2 and NH_3 .

Anaerobic microorganisms

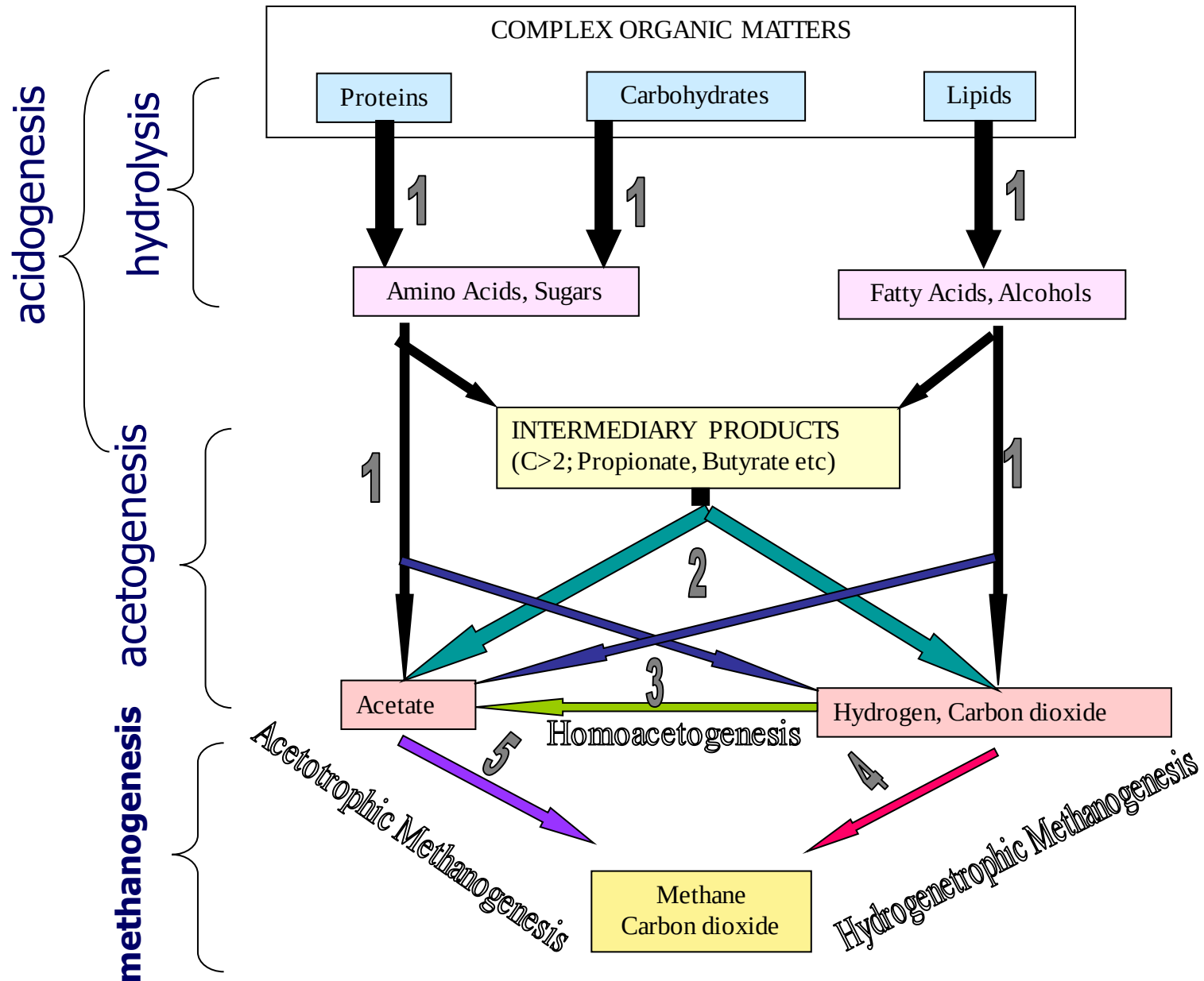
Organic materials + Nutrients $\longrightarrow$ *$CH_4 + CO_2 + NH_3 + \text{Biomass}$*

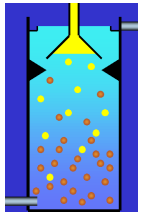
Anaerobic processes

Anaerobic fermentation

Anaerobic respiration

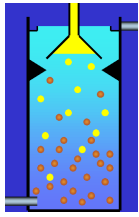
Overview Anaerobic Biodegradation



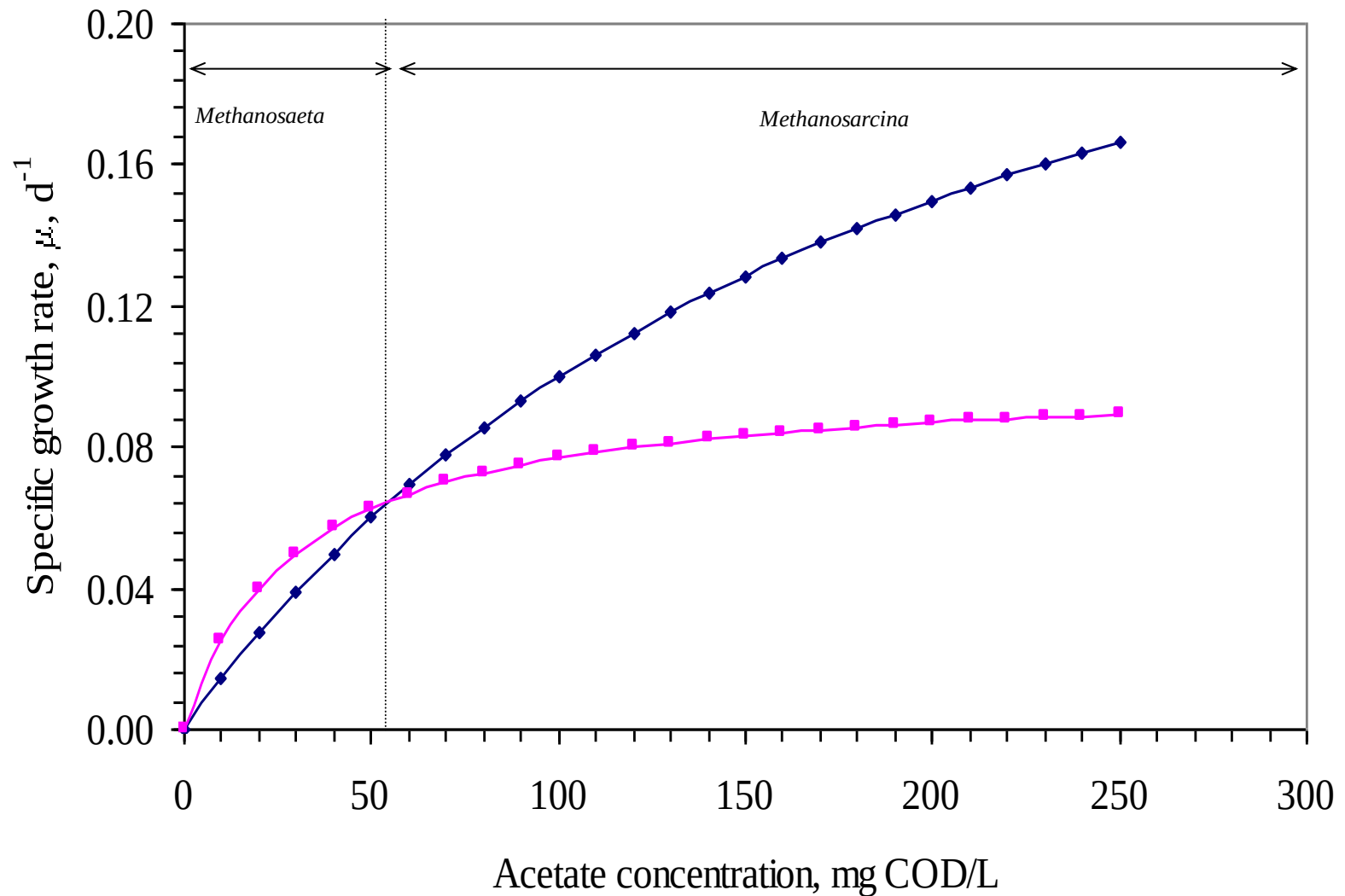


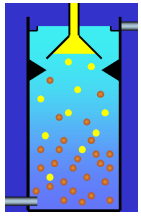
Kinetic Parameters Anaerobes

	Doubling Time days	Cell Yield g VSS g ⁻¹ COD	Cell Activity g COD g ⁻¹ VSS d ⁻¹	ks mM
Active Sludge (sugar)				
Aerobic Bacteria	0.030	0.40	57.8	0.25
Acidification (sugar)				
Fermentative Bacteria	0.125	0.14	39.6	ND
Acetogenesis (fatty acids)				
Acetogenic Bacteria	3.5	0.03	6.6	0.4
Methanogenesis				
Autotrophic (H ₂)	0.5	0.07	19.6	0.004
Acetoclastic (acetate)				
Methanosarcina	1.5	0.04	11.6	5.0
Methanosaete	7.0	0.02	5.0	0.3

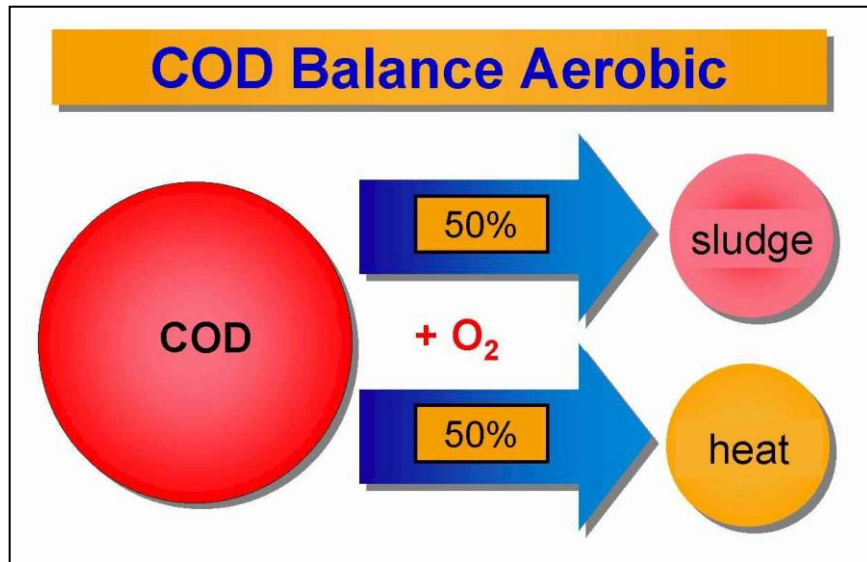


Growth kinetics of *Methanosarcina* and *Methanosaeta*

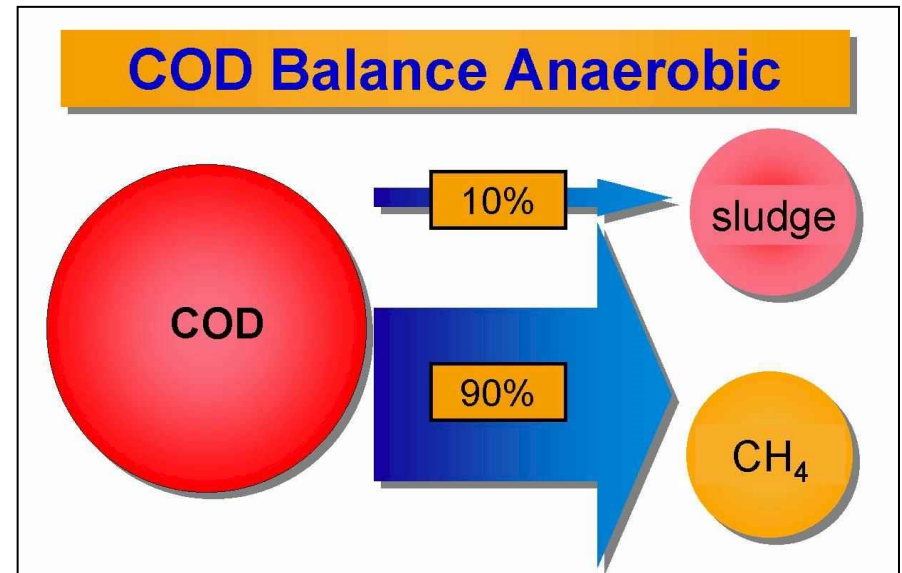




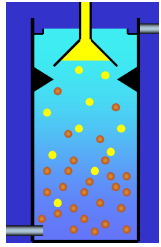
COD Balance Aerobic Biodegradation



COD Balance Anaerobic Biodegradation



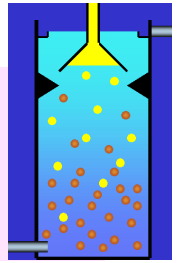
Essential conditions for efficient anaerobic treatment



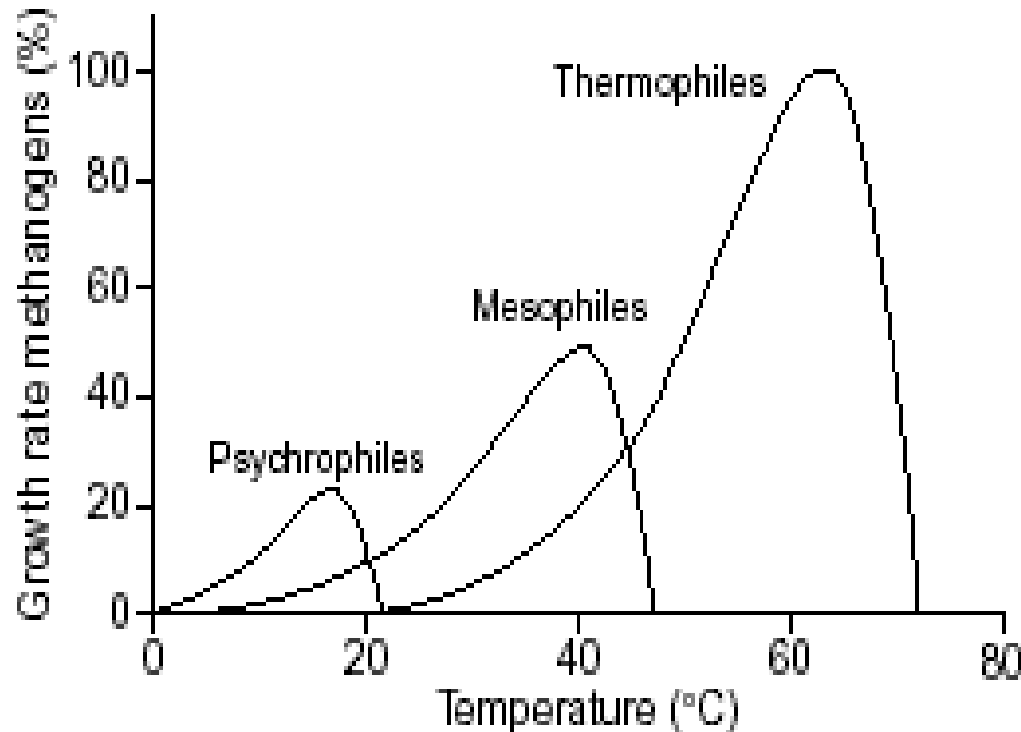
- Avoid excessive air/O₂ exposure
- No toxic/inhibitory compounds present in the influent
- Maintain pH between 6.8 –7.2
- Sufficient alkalinity present
- Temperature around mesophilic range (30-38 °C)
- Enough nutrients (N & P) and trace metals especially, Fe, Co, Ni, etc.
COD:N:P = 350:7:1 (for highly loaded system) 1000:7:1 (lightly loaded system)

Environmental factors

The successful operation of anaerobic reactor depends on maintaining the environmental factors close to the comfort of the microorganisms involved in the process.



Temperature

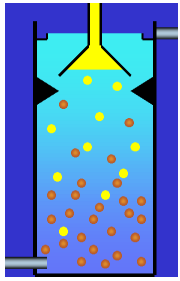


Psychrophilic (5 - 15°C)

Mesophilic (35 – 40 °C)

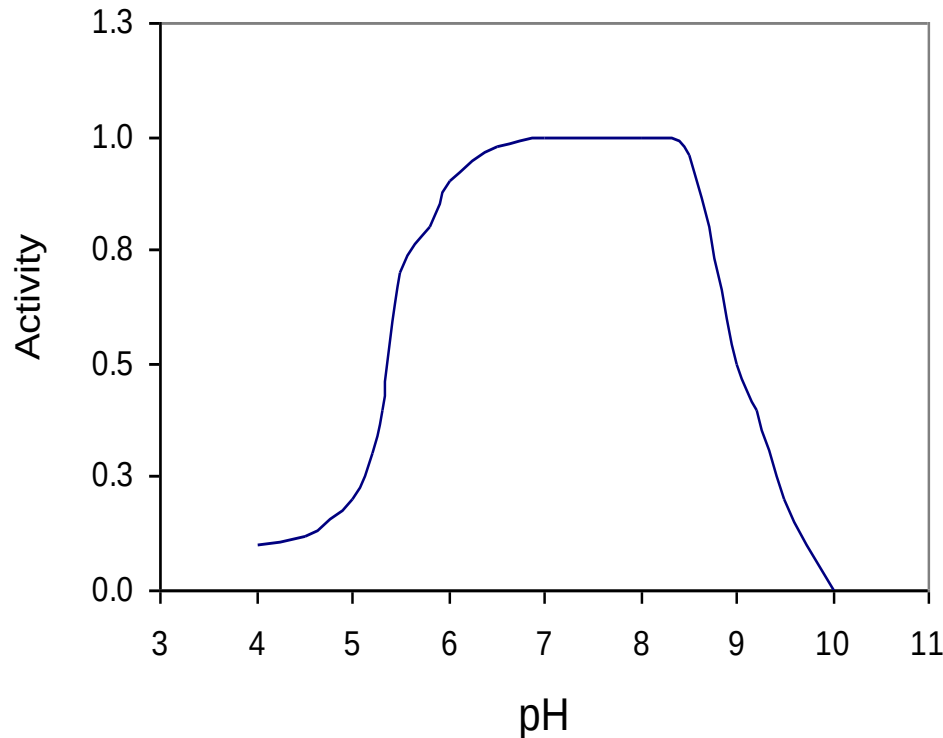
Thermophilic (50-55 °C)

Rule of thumb: Rate of a reaction doubles for every 10 degree rise in temperature



pH

Relative activity of methanogens to pH

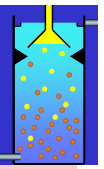


pH range for acidogens is 5.5 – 6.5

Methanogenesis 7.8 – 8.2.

operating pH for combined cultures is 6.8-7.4 with neutral pH being the optimum

Since methanogenesis is considered as a rate limiting step, It is necessary to maintain the reactor pH close to neutral.



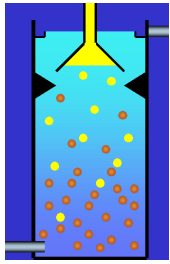
Nutrients and trace metals

All microbial processes including anaerobic process requires macro (N, P and S) and micro (trace metals) nutrients in sufficient concentration to support biomass synthesis. In addition to N and P, anaerobic microorganisms especially methanogens have specific requirements of trace metals such as Ni, Co, Fe, Mo, Se etc. The nutrients and trace metals requirements for anaerobic process are much lower as only 4 - 10% of the COD removed is converted biomass.

COD:N:P = 350:7:1

Inhibition/Toxicity

The toxicity is caused by the substance present in the influent waste or byproducts of the metabolic activities. Ammonia, heavy metals, halogenated compounds, cyanide etc. are the examples of the former type whereas ammonia, sulfide, VFAs belong to latter group.



Comparison between anaerobic and aerobic processes

Anaerobic

Organic loading rate:

High loading rates: 10-40 kg COD/m³-day
(for high rate reactors, e.g. AF, UASB, E/FBR)

Biomass yield:

Low biomass yield: 0.05-0.15 kg VSS/kg COD
(biomass yield is not constant but depends on types of substrates metabolized)

Specific substrate utilization rate:

High rate: 0.75-1.5 kg COD/kg VSS-day

Start-up time:

Long start-up: 1-2 months for mesophilic
: 2-3 months for thermophilic

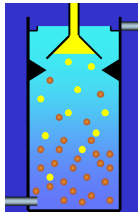
Aerobic

Low loading rates: 0.5-1.5 kg COD/m³-day
(for activated sludge process)

High biomass yield: 0.37-0.46 kg VSS/kg COD
(biomass yield is fairly constant irrespective of types of substrates metabolized)

Low rate: 0.15-0.75 kg COD/kg VSS-day

Short start-up: 1-2 weeks



Continue.....

Anaerobic

SRT:

Longer SRT is essential to retain the slow growing methanogens within the reactor.

Microbiology:

Anaerobic process is multi-step process and diverse group of microorganisms degrade the organic matter in a sequential order.

Environmental factors:

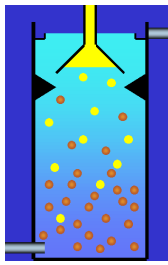
The process is highly susceptible to changes in environmental conditions.

Aerobic

SRT of 4-10 days is enough in case of activated sludge process.

Aerobic process is mainly a one-species phenomenon.

The process is less susceptible to changes in environmental conditions.



Advantage of anaerobic process

1. Less energy requirement as no aeration is needed

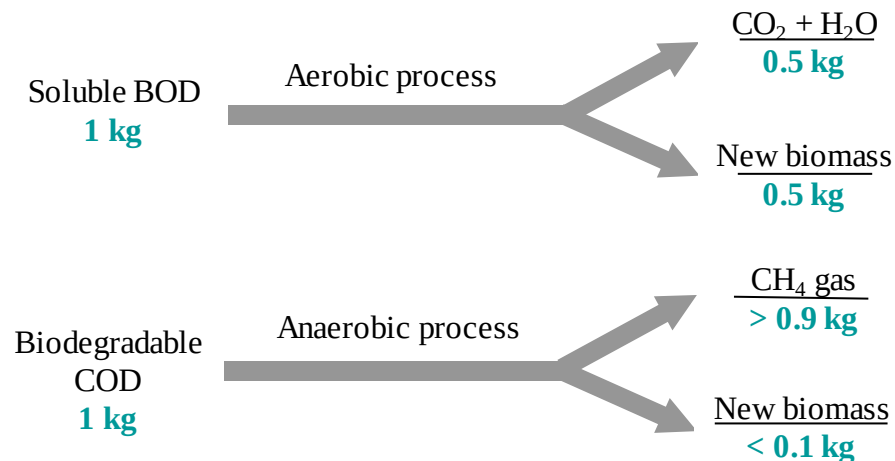
0.5-0.75 kwh energy is needed for every 1 kg of COD removal by aerobic process

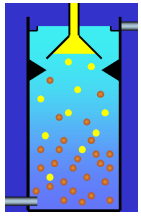
2. Energy generation in the form of methane gas

1.16 kwh energy is produced for every 1 kg of COD removal by anaerobic process

3. Less biomass (sludge) generation

Anaerobic process produces only 20% of sludge that of aerobic process





4. Less nutrients (N & P) requirement

Lower biomass synthesis rate also implies less nutrients requirement : 20% of aerobic

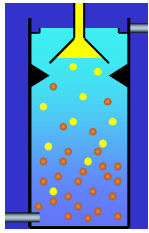
5. Application of higher organic loading rate

Organic loading rates of 5-10 times higher than that of aerobic processes are possible

6. Space saving

Application of higher loading rate requires smaller reactor volume thereby saving the land requirement

7. Ability to transform several hazardous solvents including chloroform, trichloroethylene and trichloroethane to an easily degradable form



Limitations of anaerobic processes

1. Long start-up time

Because of lower biomass synthesis rate, it requires longer start-up time to attain a biomass concentration.

2. Long recovery time

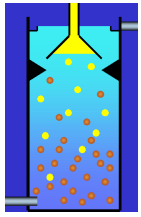
If an anaerobic system subjected to disturbances either due to biomass wash-out, toxic substances or shock loading, it may take longer time for the system to return to normal operating condition.

3. Specific nutrients/trace metal requirements

Anaerobic microorganisms especially methanogens have specific nutrients e.g. Fe, Ni, and Co requirement for optimum growth.

4. More susceptible to changes in environmental conditions

Anaerobic microorganisms especially methanogens are prone to changes in conditions such as temperature, pH, etc.



5. Treatment of sulfate rich wastewater

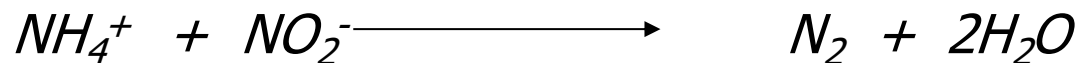
The presence of sulfate not only reduces the methane yield due to substrate competition but also inhibits the methanogens due to sulfide production.

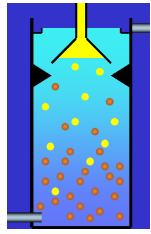
6. Effluent quality of treated wastewater

The minimum substrate concentration (S_{min}) from which microorganisms are able to generate energy for their growth and maintenance is much higher for anaerobic treatment system. Owing to this fact, anaerobic processes may not be able to degrade the organic matter to the level meeting the discharge limits for ultimate disposal.

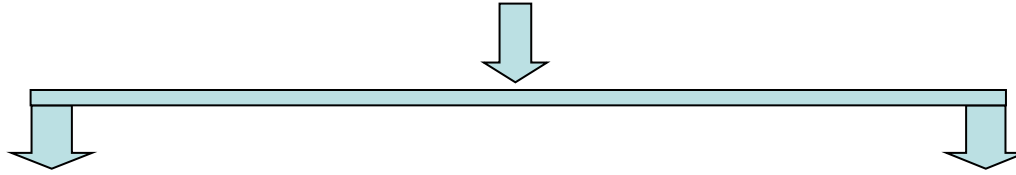
7. Treatment of high protein & nitrogen containing wastewater

The anaerobic degradation of proteins produces amines which are no longer be degraded anaerobically. Similarly nitrogen remains unchanged during anaerobic treatment. Recently, a process called ANAMMOX (ANaerobic AMMonium OXididation) has been developed to anaerobically oxidize NH_4^+ to N_2 in presence of nitrite.

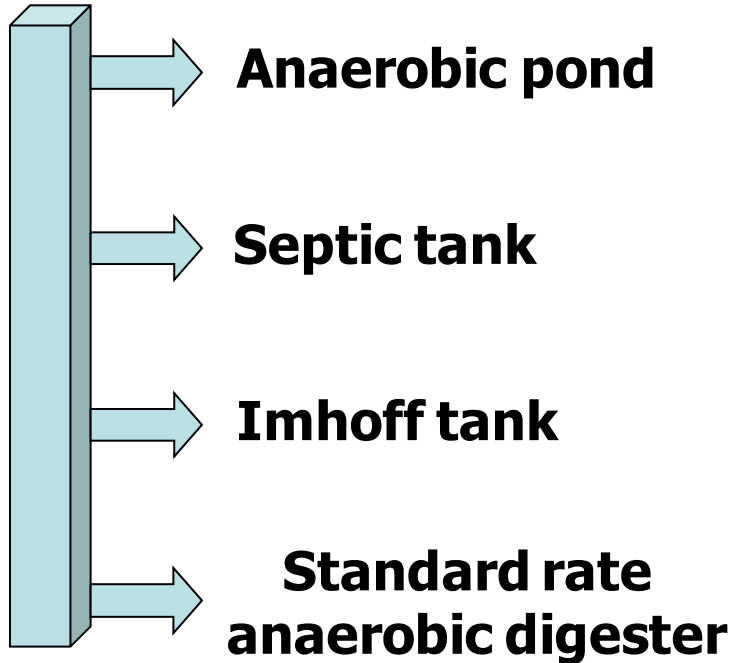




Types of anaerobic reactors

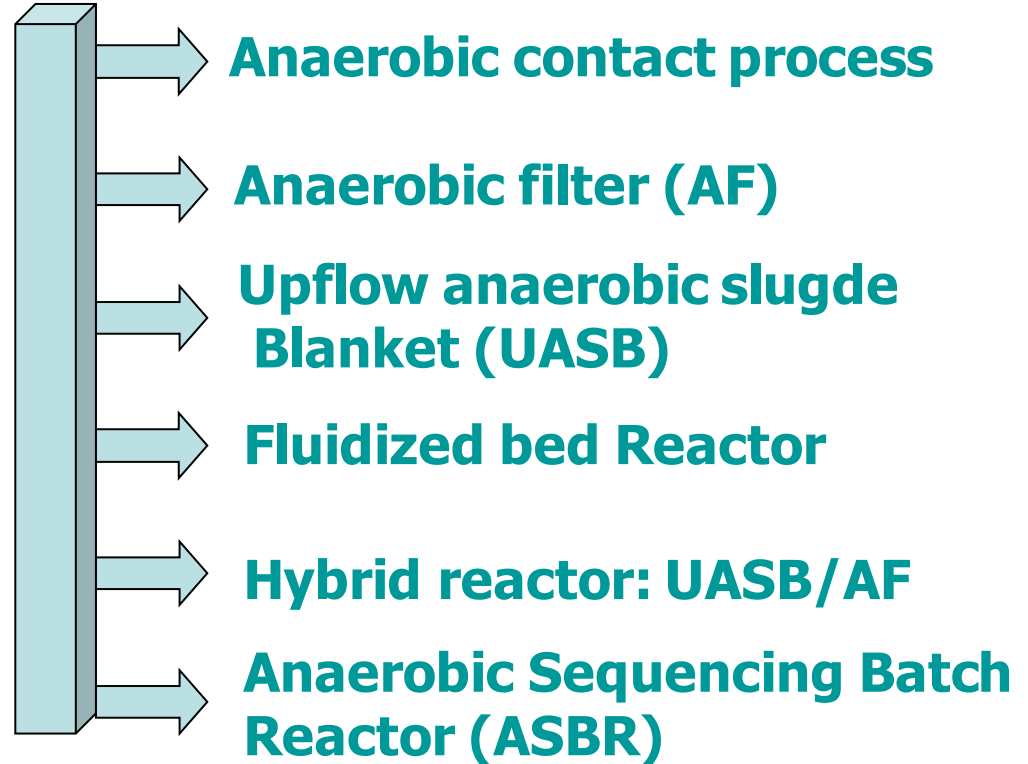


Low rate anaerobic reactors

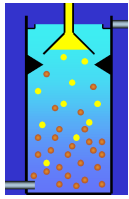


Slurry type bioreactor, temperature, mixing, SRT or other environmental conditions are not regulated. Loading of 1-2 kg COD/m³-day.

High rate anaerobic reactors



Able to retain very high concentration of active biomass in the reactor. Thus extremely high SRT could be maintained irrespective of HRT. Load 5-20 kg COD/m³-d
COD removal efficiency : 80-90%



Upflow Anaerobic Sludge Blanket (UASB)



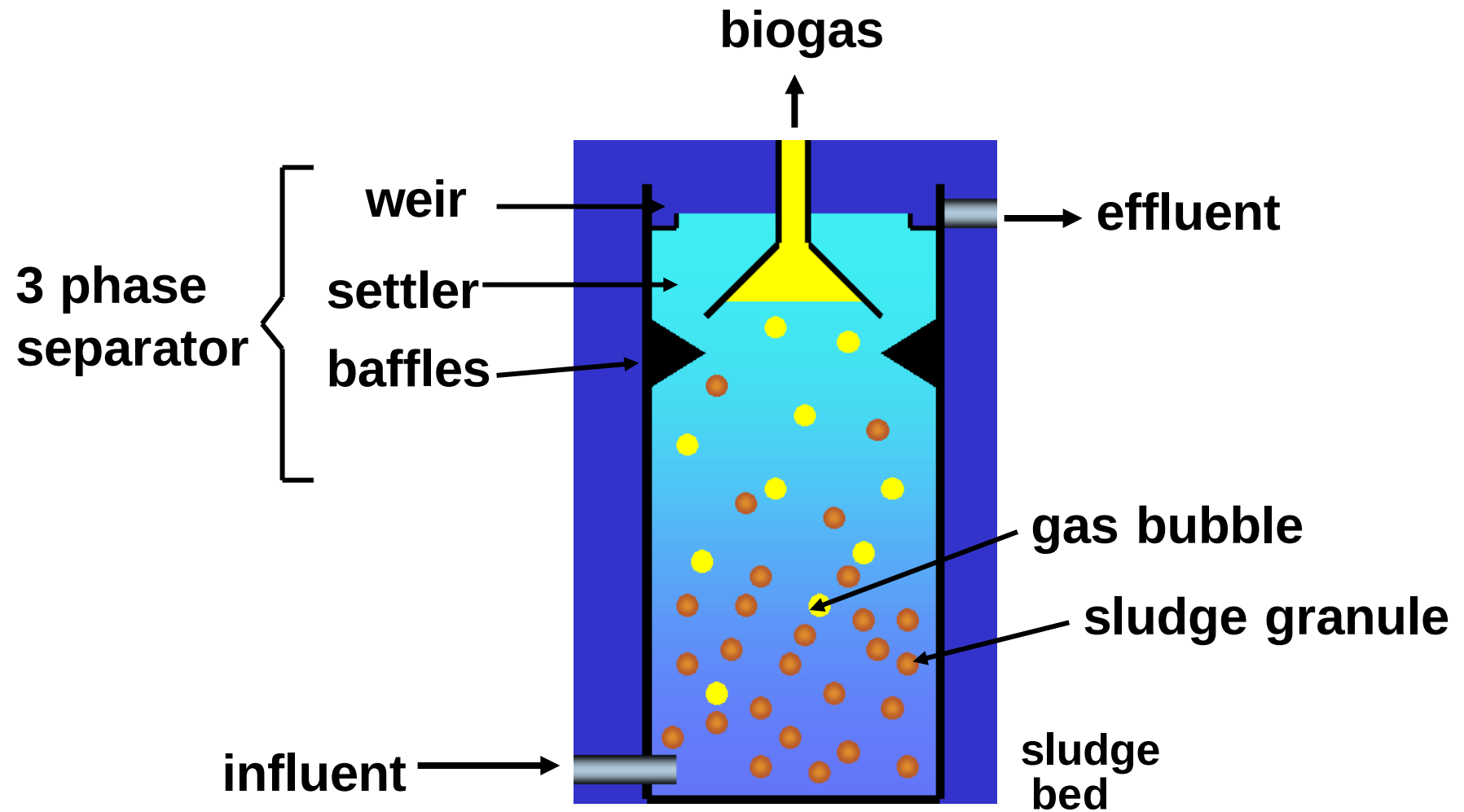
UASB was developed in 1970s by Lettinga in the Netherlands.

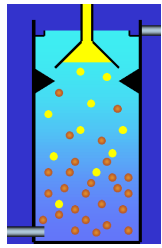
UASB is essentially a suspended growth system in which proper hydraulic and organic loading rate is maintained in order to facilitate the dense biomass aggregation known as granulation. The size of granules is about 1-3 mm diameter. Since granules are bigger in size and heavier, they will settle down and retain within the reactor. The concentration of biomass in the reactor may become as high as 50 g/L. Thus a very high SRT can be achieved even at very low HRT of 4 hours.



The granules consist of hydrolytic bacteria, acidogen/acetogens and methanogens. Carbohydrate degrading granules show layered structure with a surface layer of hydrolytic/fermentative Acidogens. A mid-layer comprising of syntrophic colonies and an interior with acetogenic methanogens.

Upward-flow Anaerobic Sludge Blanket





Anaerobic Sludge Granules

Physical:

dense compact biofilms

high settleability

high mechanical strength

Microbial:

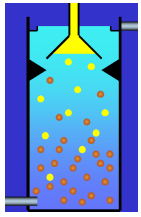
balanced microbial community

syntrophic partners closely associated

high methanogenic activity
(0.5 to 2.0 g COD/g VSS.d)

protection from toxic shock





The spaghetti theory of granulation

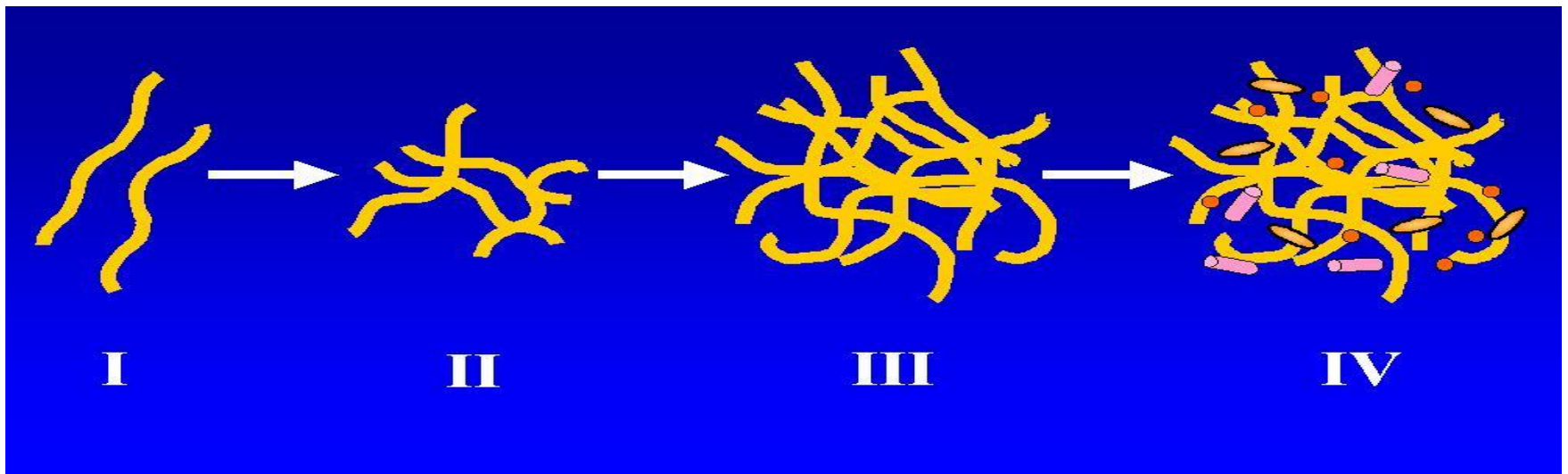
proposed by Dr. W. Wiegant

I) disperse methanogens (filamentous *Methanosaeta*

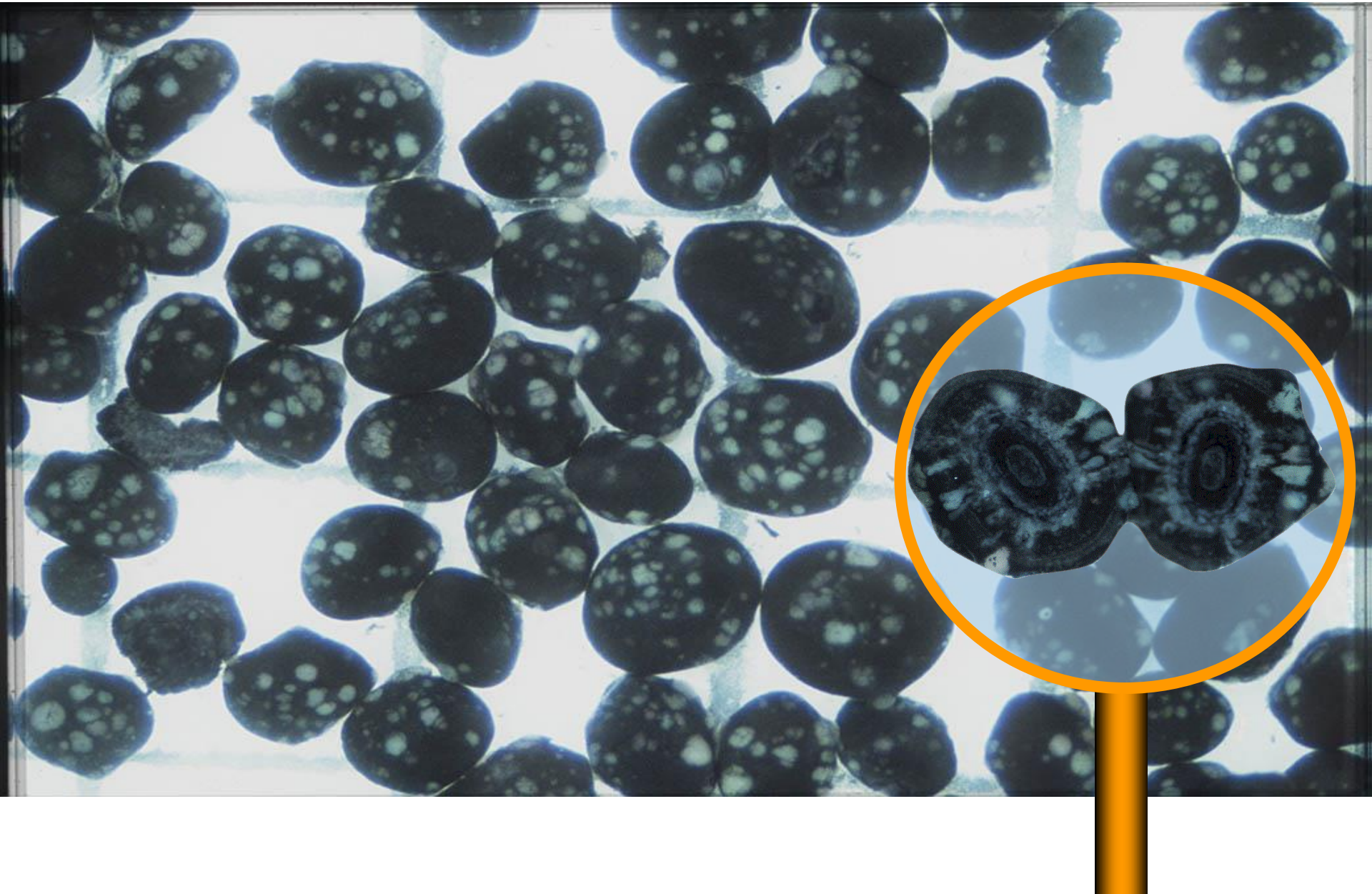
II) floccule formation via entanglement

III) pellet formation ("spaghetti balls");

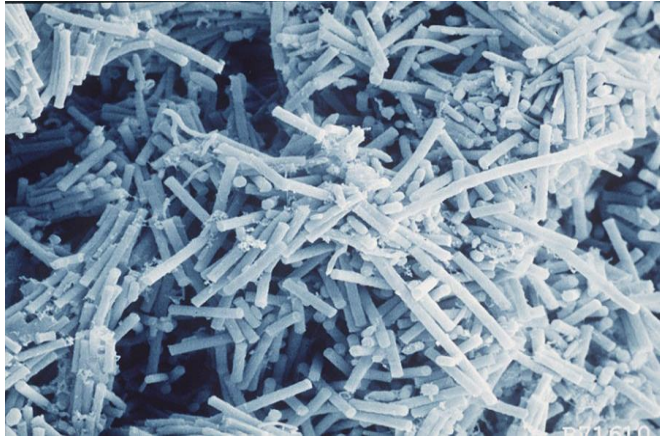
IV) mature granules,
with attachment of other anaerobic microorganisms onto the pellet.



Anaerobic Sludge Granules (close up)



Anaerobic Sludge Granules (SEM)

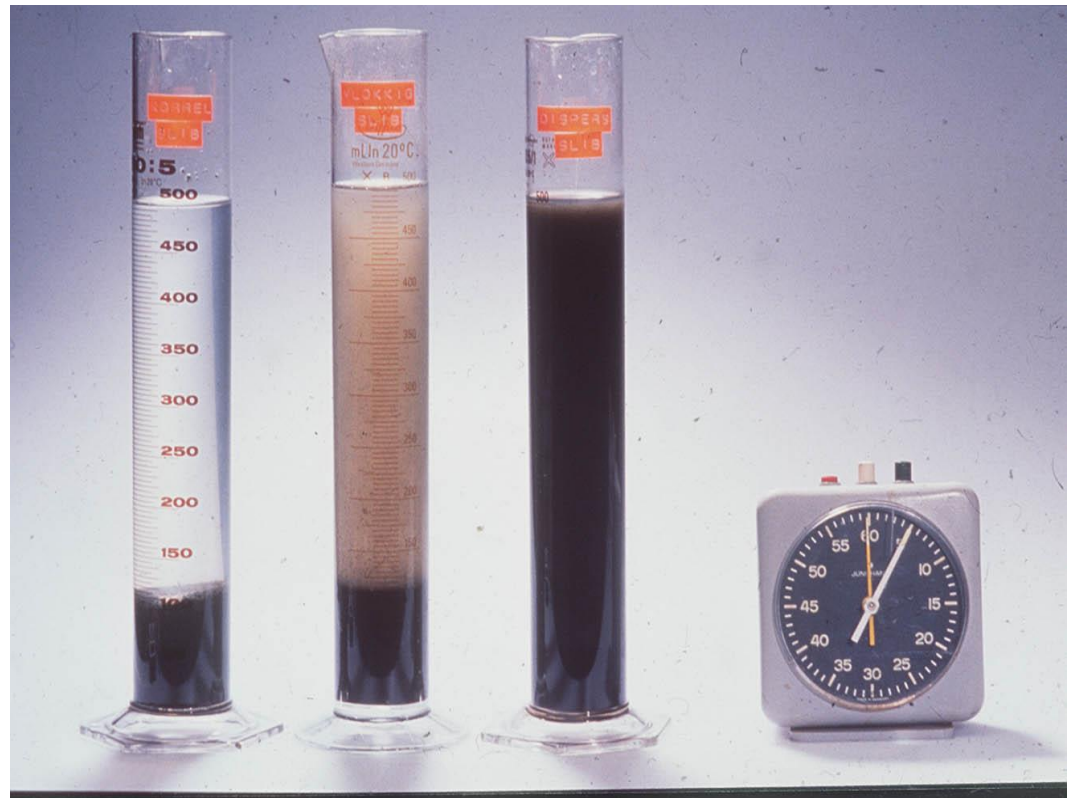


Acetate as Substrate
(*Methanosaeta*)

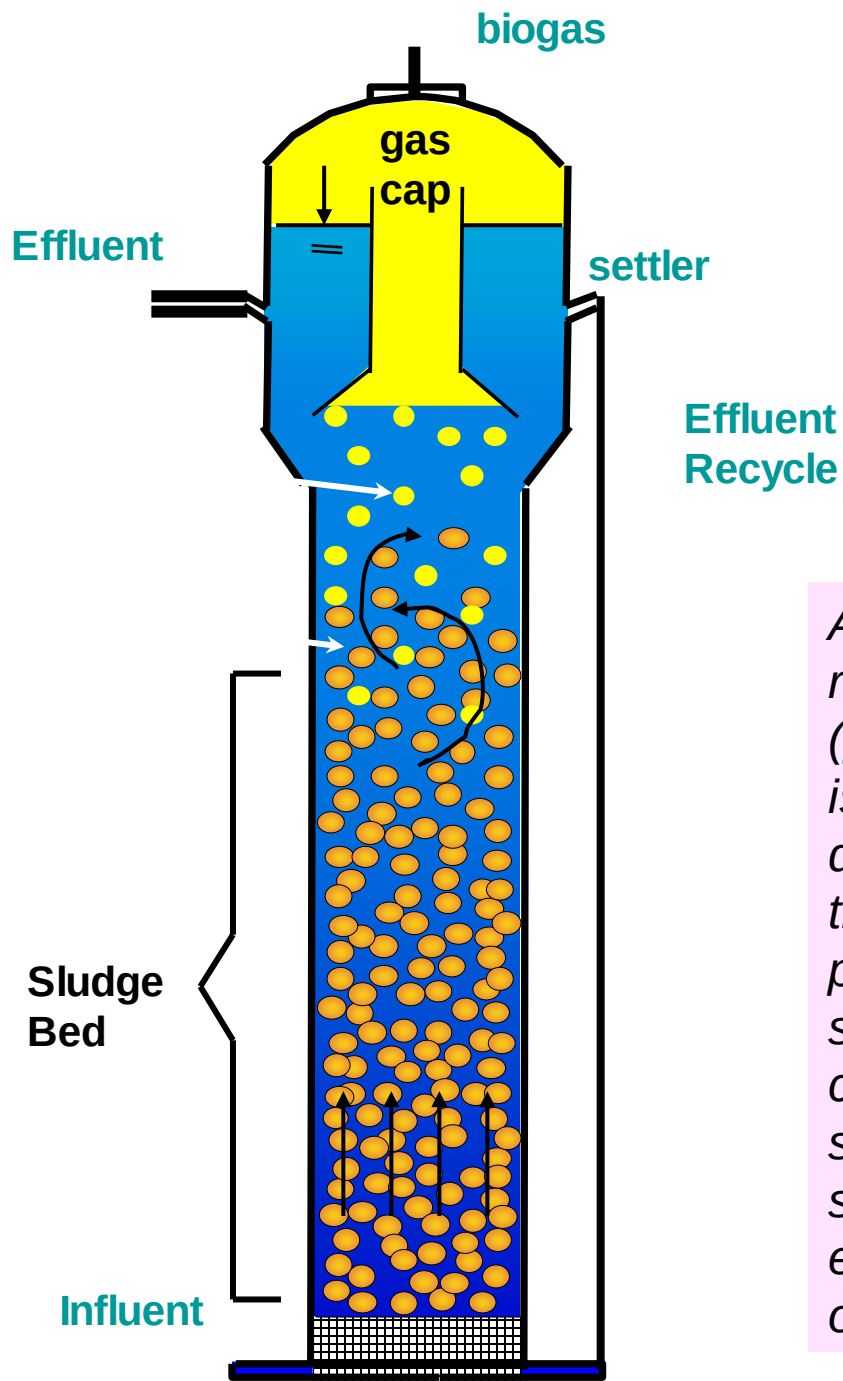


Sucrose as Substrate (mixed
culture)

Anaerobic Sludge Granules (settling)

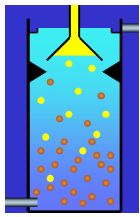


granular flocculent dispersed



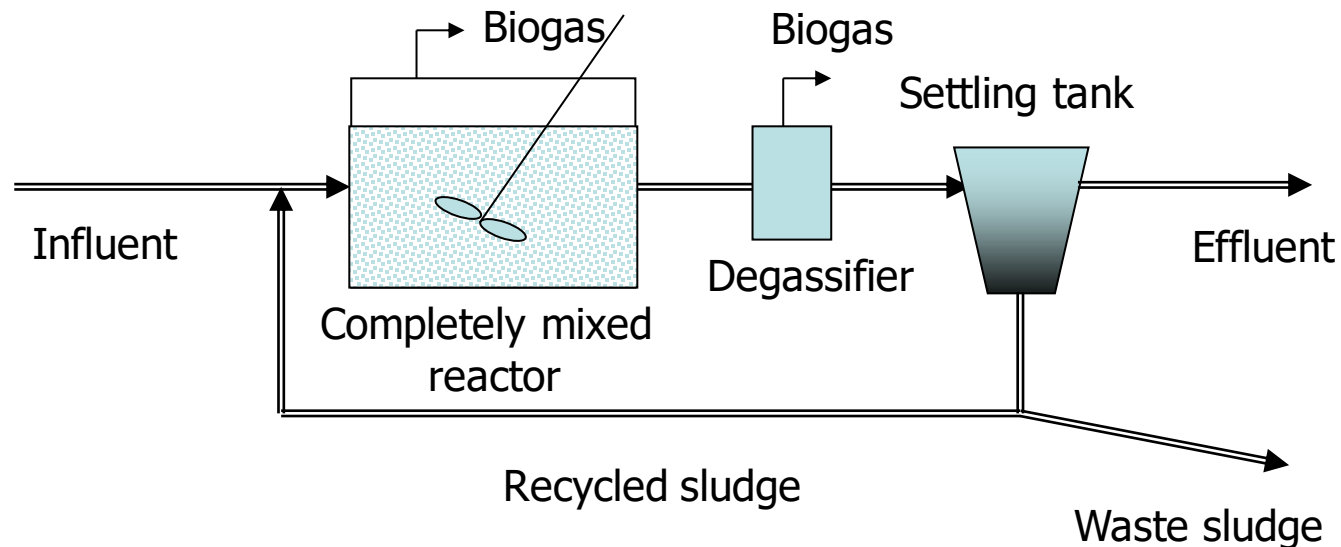
Expanded Granular Sludge Bed

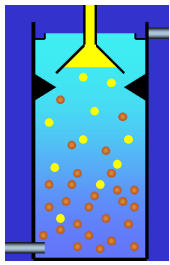
An expanded granular sludge bed (EGSB) reactor is a variant of the UASB concept ([Kato et al. 1994](#)). The distinguishing feature is that a faster rate of upward-flow velocity is designed for the wastewater passing through the sludge bed. The increased flux permits partial expansion (fluidization) of the granular sludge bed, improving wastewater-sludge contact as well as enhancing segregation of small inactive suspended particle from the sludge bed. The increased flow velocity is either accomplished by utilizing tall reactors, or by incorporating an effluent recycle



Anaerobic contact process (ACP)

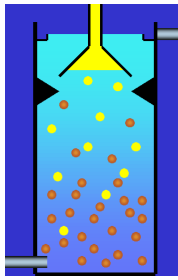
Anaerobic contact process is essentially an anaerobic activated sludge process. It consists of a completely mixed reactor followed by a settling tank. The settled biomass is recycled back to the reactor. Hence ACP is able to maintain high concentration of biomass in the reactor and thus high SRT irrespective of HRT. Degassifier allows the removal of biogas bubbles (CO_2 , CH_4) attached to sludge which may otherwise float to the surface.



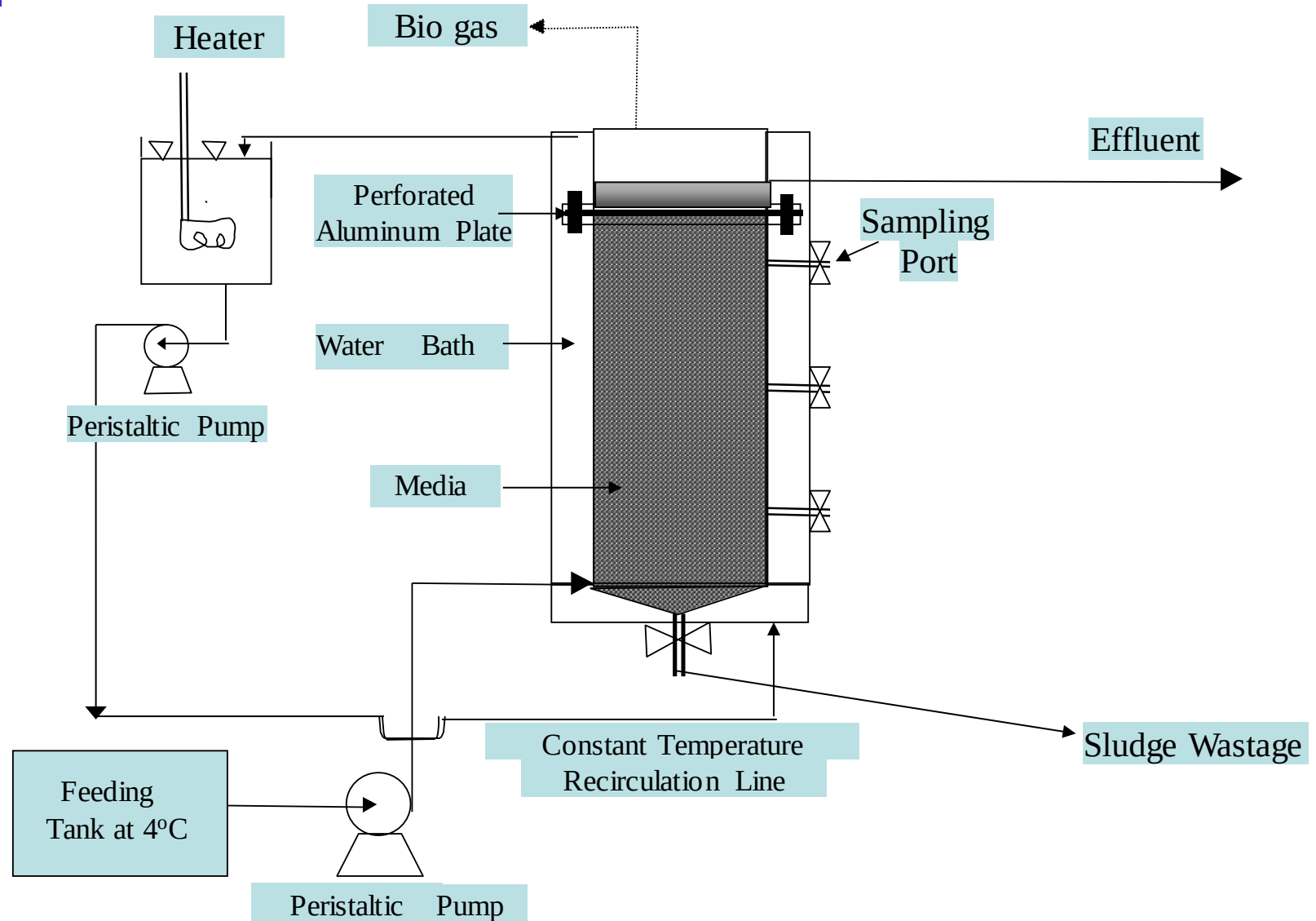


Anaerobic filter

- **Anaerobic filter:** Young and McCarty in the late 1960s for treat dilute soluble organic wastes.
- The filter was filled with rocks similar to the trickling filter.
- Wastewater distributed across the bottom and the flow was in the upward direction through the bed of rocks
- Whole filter submerged completely
 - Anaerobic microorganisms accumulate within voids of media (rocks or other plastic media)
- The media retain or hold the active biomass within the filter
- The non-attached biomass within the interstices forms a bigger flocs of granular shape due to rising gas bubble/liquid
- Non-attached biomass contributes significantly to waste treatment
- Attached biomass not be a major portion of total biomass.
- 64% attached and 36% non-attached



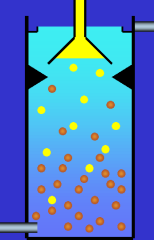
Upflow Anaerobic Filter



Originally, rocks were employed as packing medium in anaerobic filter. But due to very low void volume (40-50%), serious clogging problem was witnessed. Now, many synthetic packing media made up of plastics, ceramic tiles of different configuration have been used in anaerobic filters. The void volume in these media ranges from 85-95 %. Moreover, these media provide high specific surface area typically $100 \text{ m}^2/\text{m}^3$ or above which enhance biofilm growth.



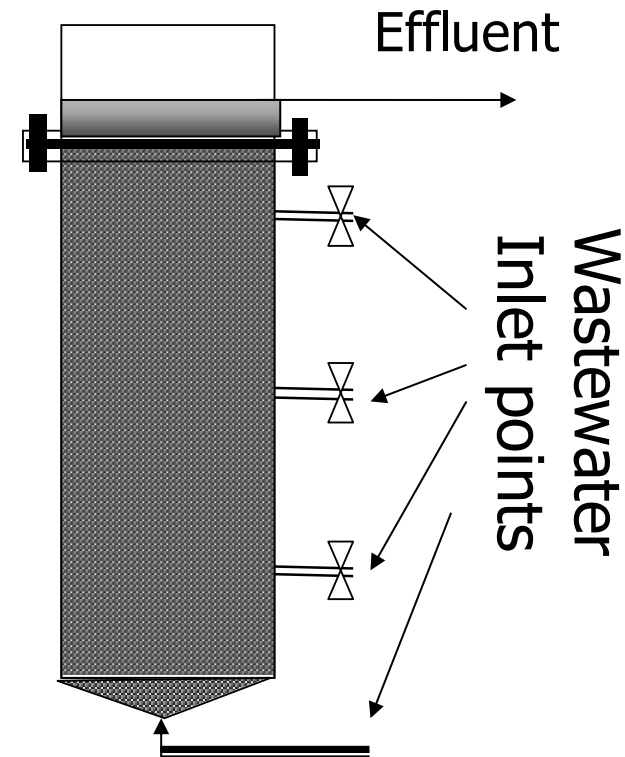
Since anaerobic filter is able to retain high biomass, long SRT could be maintained. Typically HRT varies from 0.5 – 4 days and the loading rates varies from 5 - 15 kg COD/ m^3 -day. Biomass wastage is generally not needed and hydrodynamic conditions play important role in biomass retention within the void space

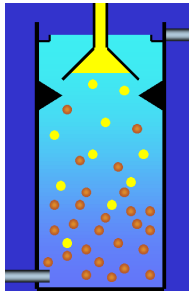


Multi-fed Up Flow Anaerobic Filter (MUAF)

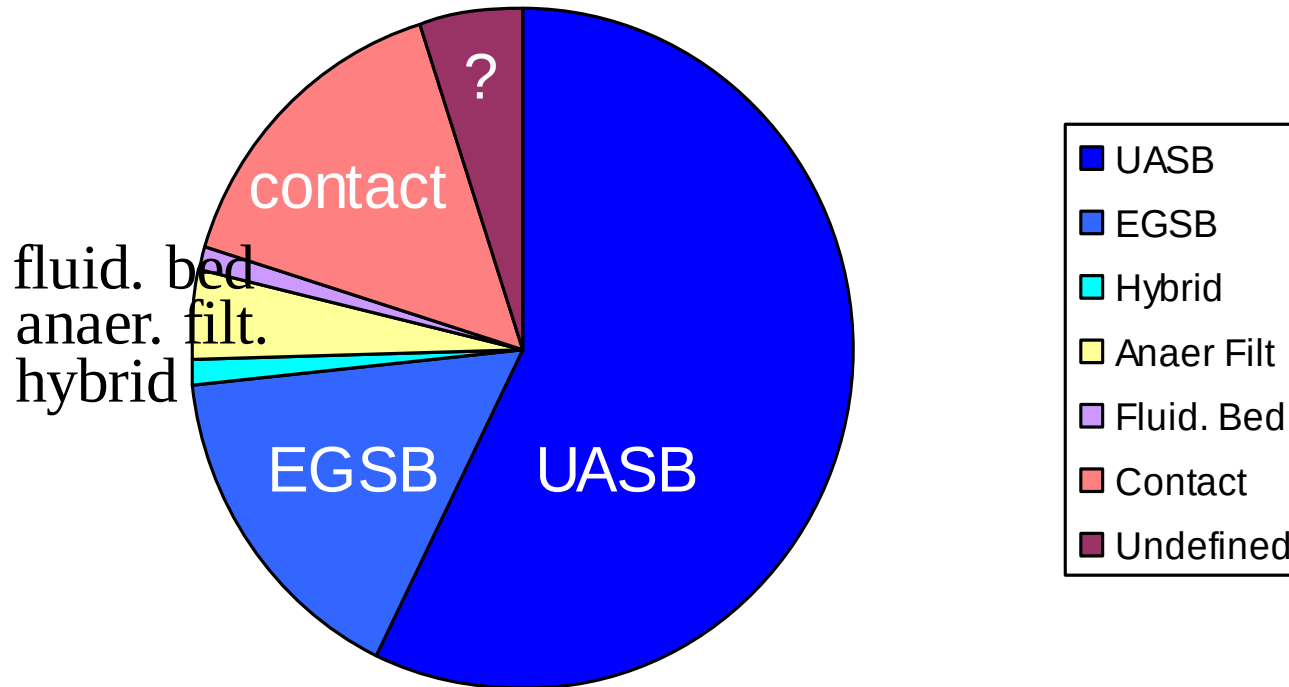
Waste is fed through several points along the depth of filter. Such feeding strategy has unique benefits:

1. Homogeneity in biomass distribution
2. Maintenance of completely mixed regime thus preventing short - circuiting and accumulation of VFA.
3. Uniform substrate concentration within the reactor and prevent heavy biomass growth at bottom thus avoids clogging
4. Effective utilization of whole filter bed

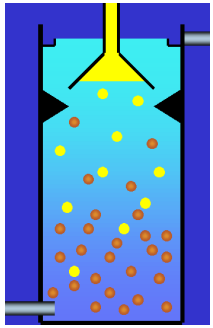




Market-Share Granular Sludge Reactors



EGSB+UASB = 72%



OPTIONS FOR POST-TREATMENT OF ANAEROBIC REACTOR EFFLUENTS

Beginning with a typical municipal raw wastewater, this level of treatment will generally result in an “enhanced primary” effluent quality, intermediate between primary and secondary (between 30-70 mg/l for BOD₅). Post-treatment should be designed to improve the effluent quality in the following parameters.

- *pathogen contamination (measured by the index of *E. coli*);*
- *residual organic material (COD/BOD₅);*
- *oxygen demand from the reduced forms of N and S;*
- *residual suspended solids (TSS)*
- *inorganic N and P (nutrients)*

basic types of post treatment processes are:

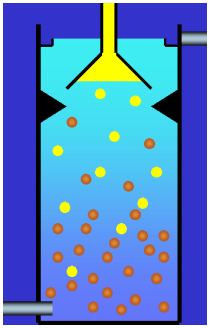
Pond systems

Constructed wetlands

duckweed

Mechanical aerated post treatment





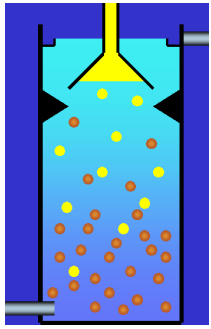
Indian Scenario

The government of India has made a major commitment to anaerobic treatment technology in its national river basin improvement program. As of 1996, thirteen new anaerobic treatment plants, with an aggregate treatment capacity of over 306 MLD are under construction in India. The treatment plants described below have been in operation long enough to be able to evaluate their treatment effectiveness and their financial and economic costs and benefits:

A 5 MLD plant in Kanpur, in the state of Uttar Pradesh, built in the late 1989,

A 14 MLD plant in Mirzapur, Uttar Pradesh, based on the Kanpur pilot plant design, was commissioned in 1991,

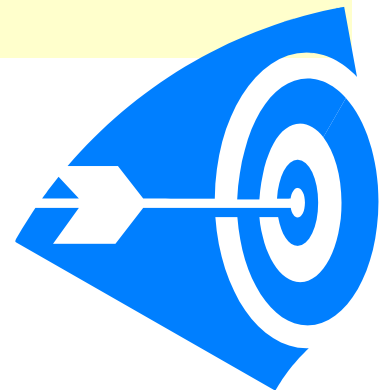
A 36 MLD plant in Kanpur reached full performance in 1994, treating a mixture of up to 75 percent municipal wastewater and 25 percent tannery effluent.

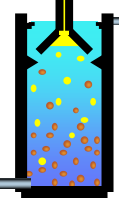


Concluding remarks

The UASB-process represents one important option for sewage purification in countries with warm climates as it meets the above mentioned basic necessities for a sustainable operation of wastewater treatment plants in developing countries like

- Low investment costs,
- Low maintenance demand,
- Good performance,
- Low sludge production
- Net energy production.





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