

chapter-3-

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Conventional wastewater treatment processes

Main goals of WW Treatment

1. Prevention of pollution to:

- **Ground water**
- **Soil**
- **marine life (aquatic life)**
- **Water bodies**
- **air**

cont== Main goals

2. **Protection of the public health**
3. **Solving asocial problems caused by the acumen of wastewater**
4. **Saving of water for reuse purpose such as:**
 - **Agriculture**
 - **Recharge**
 - **Industrial recycle**

Technical goals of wastewater

- 1. Separation of solids from liquid**
- 2. Solid stabilization**
- 3. Proper reuse and or disposal of liquid and solids**
- 4. Destroy pathogenic microorganism**

How to achieve the technical goals?

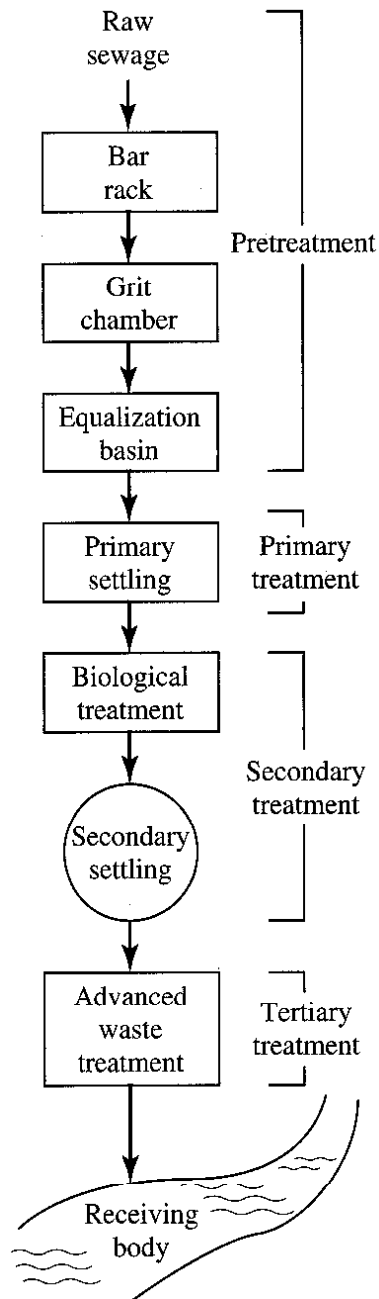


How
to
achieve
the
technical
Goals?

How to achieve the technical goals?

- ❖ **Preliminary treatment**
- ❖ **Primary treatment**
- ❖ **Secondary treatment**
- ❖ **Physical- chemical treatment**
- ❖ **Advanced wastewater treatment**
- ❖ **Des-infection**
- ❖ **Effluent discharge**

Municipal Wastewater Treatment Systems



- Pretreatment – removes materials that can cause operational problems, equalization optional
- Primary treatment – remove ~60% of solids and ~35% of BOD
- Secondary treatment – remove ~85% of BOD and solids
- Advanced treatment – varies: 95+ % of BOD and solids, N, P

Preliminary treatment

Remove solids by screening and settling

- The sewage is passed through a screen to remove large pieces of debris (e.g. sticks, stones, rags, and plastic bags).
- Next, the sewage enters a grit chamber, where the water flow is slowed just enough to allow coarse sand and gravel to settle out on the bottom.

Preliminary treatment

Screening:

- **Manual bar screen**
- **Mechanical bar screen**
- **comminutors**

Aim

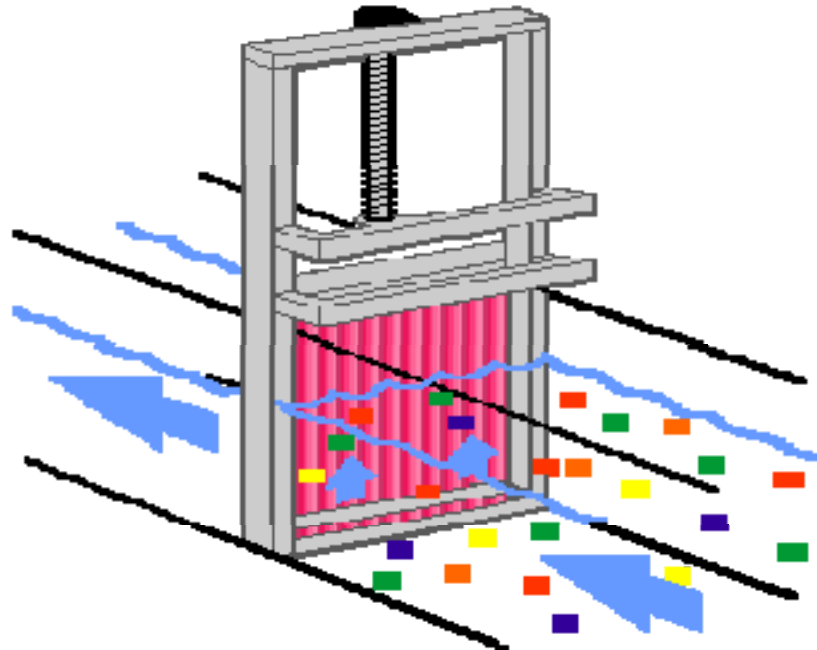


- **To remove floating materials**
- **To prevent damage to mechanical equipment such as pumps & aerators**
- **To prevent blockage of pipes**

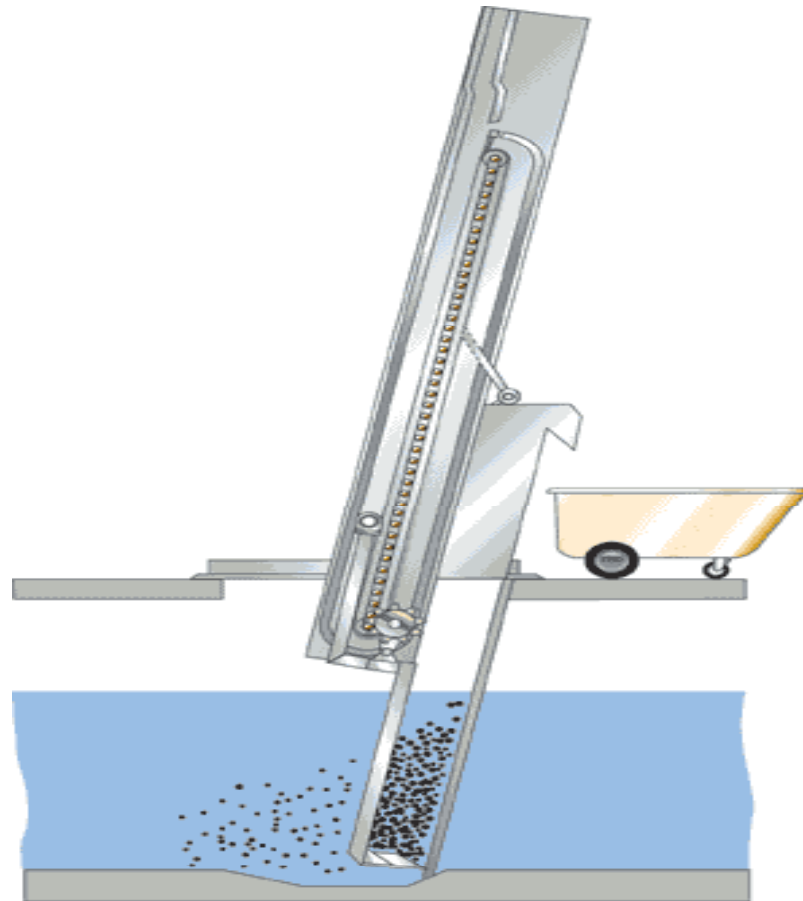
Channel width (W):

$$W = (Q/VD) * \begin{matrix} 2 & \text{---} 12\text{mm} \\ 1.67 & \text{---} 18\text{mm} \\ 1.5 & \text{---} 24\text{mm} \end{matrix}$$

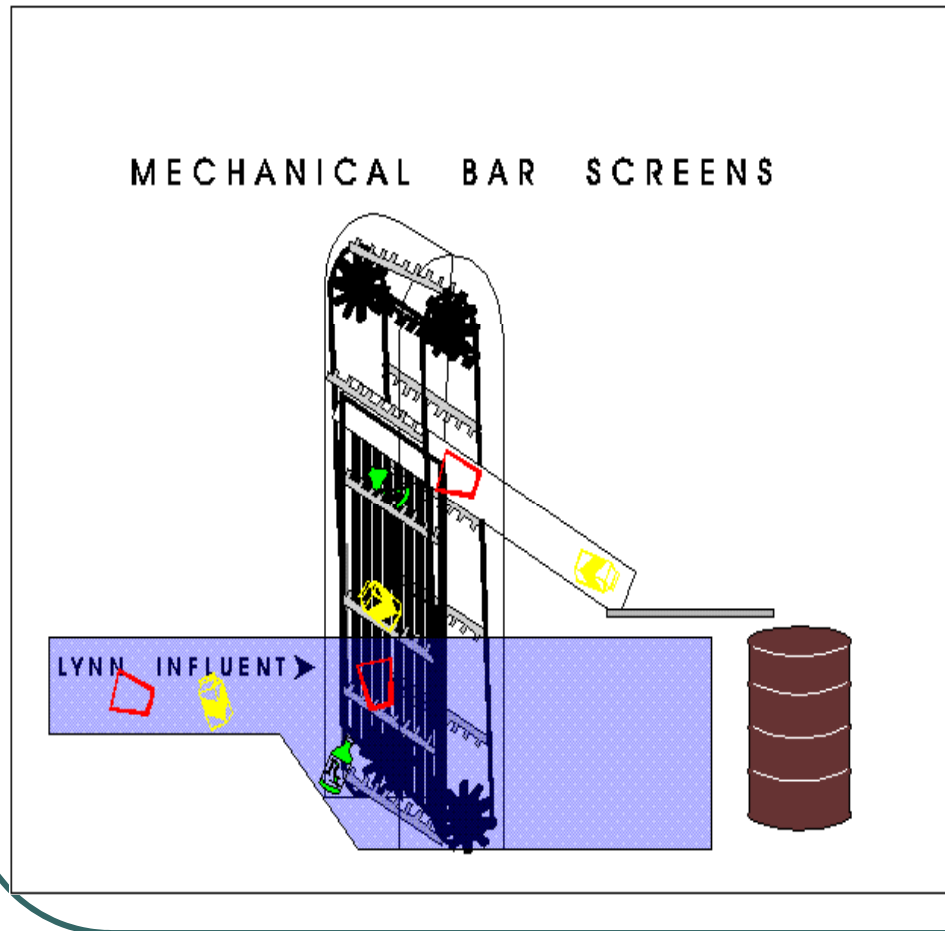
Manual bar screen



Mechanical bar screen



Mechanical bar screen



Grit removal

Aim

- To prevent abrasion & wear of mechanical equipment
- To prevent deposition of grit in pipes / channels

((It is important to remove particle with diameter >0.2 mm and to prevent organic material sedimentation))

Grit Basin



Removal grit

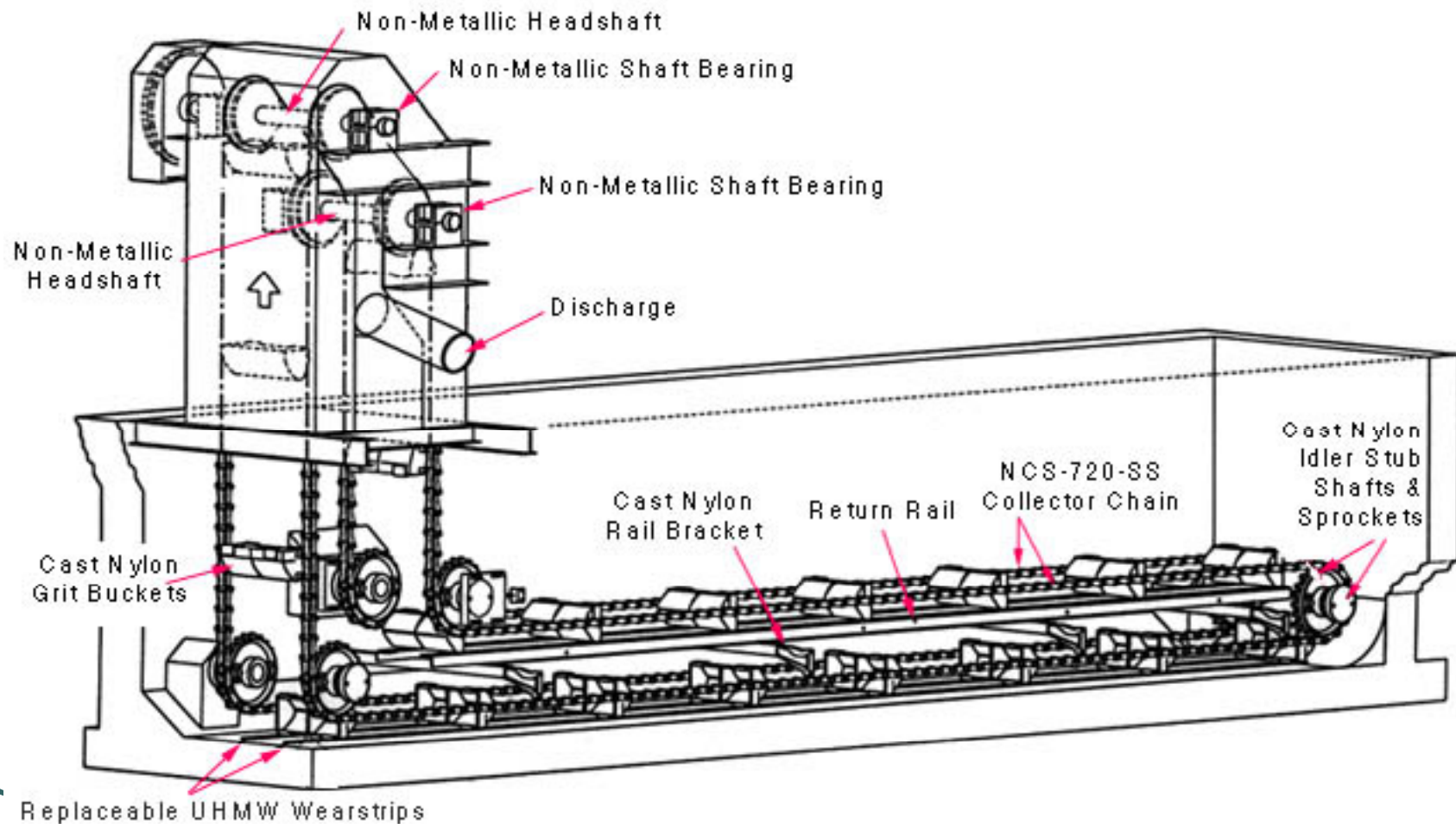
1. Grit channel:

- **Channel width = $1.5A/H$**
- **Velocity of 0.03m/s is needed for grit settling.**
- **Channel length = $20 * \text{channel depth}$**

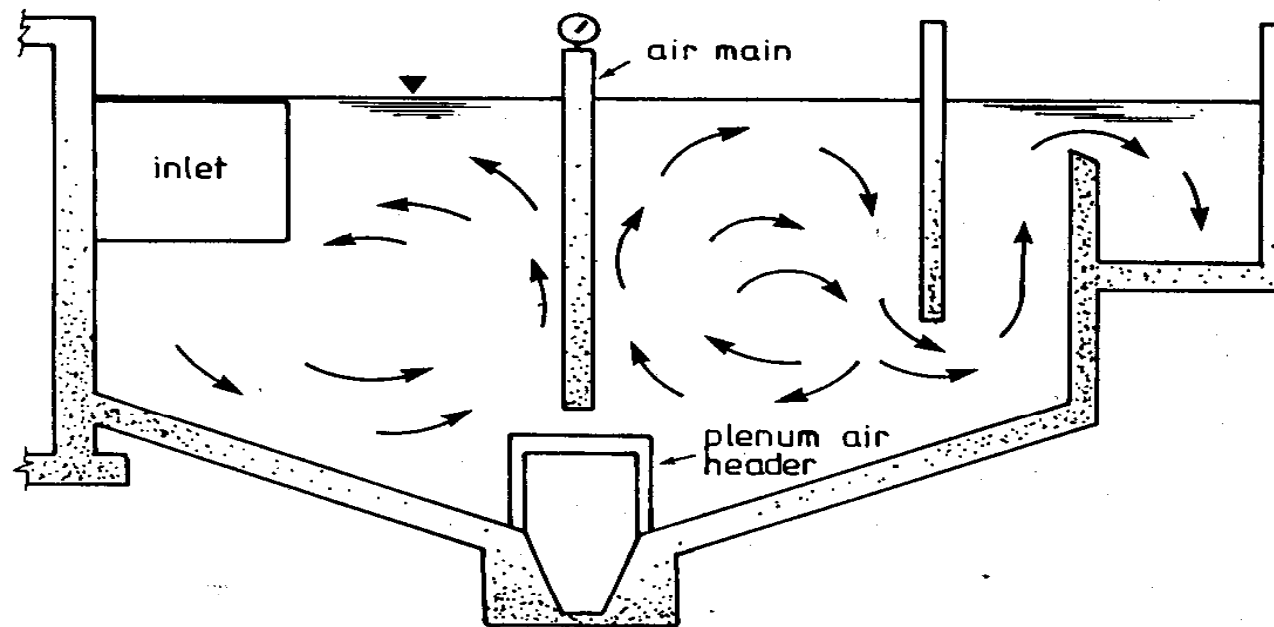
Grit removal



Grit Chambers: Velocity Controlled

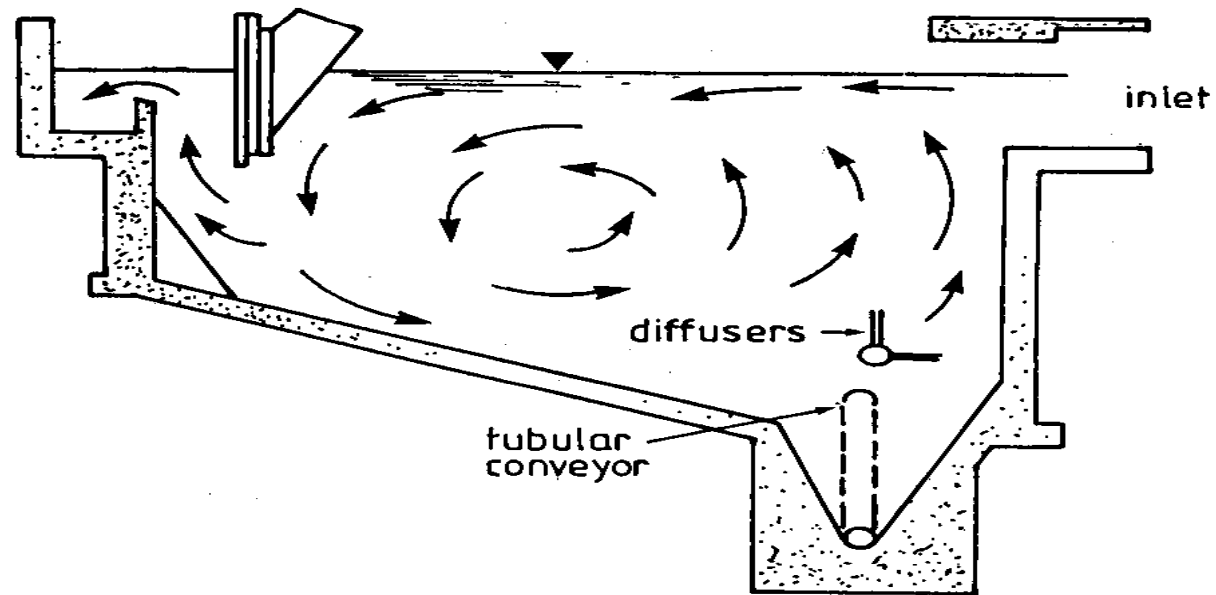


2. Aerated grit chambers



(a)

2. Aerated grit chambers



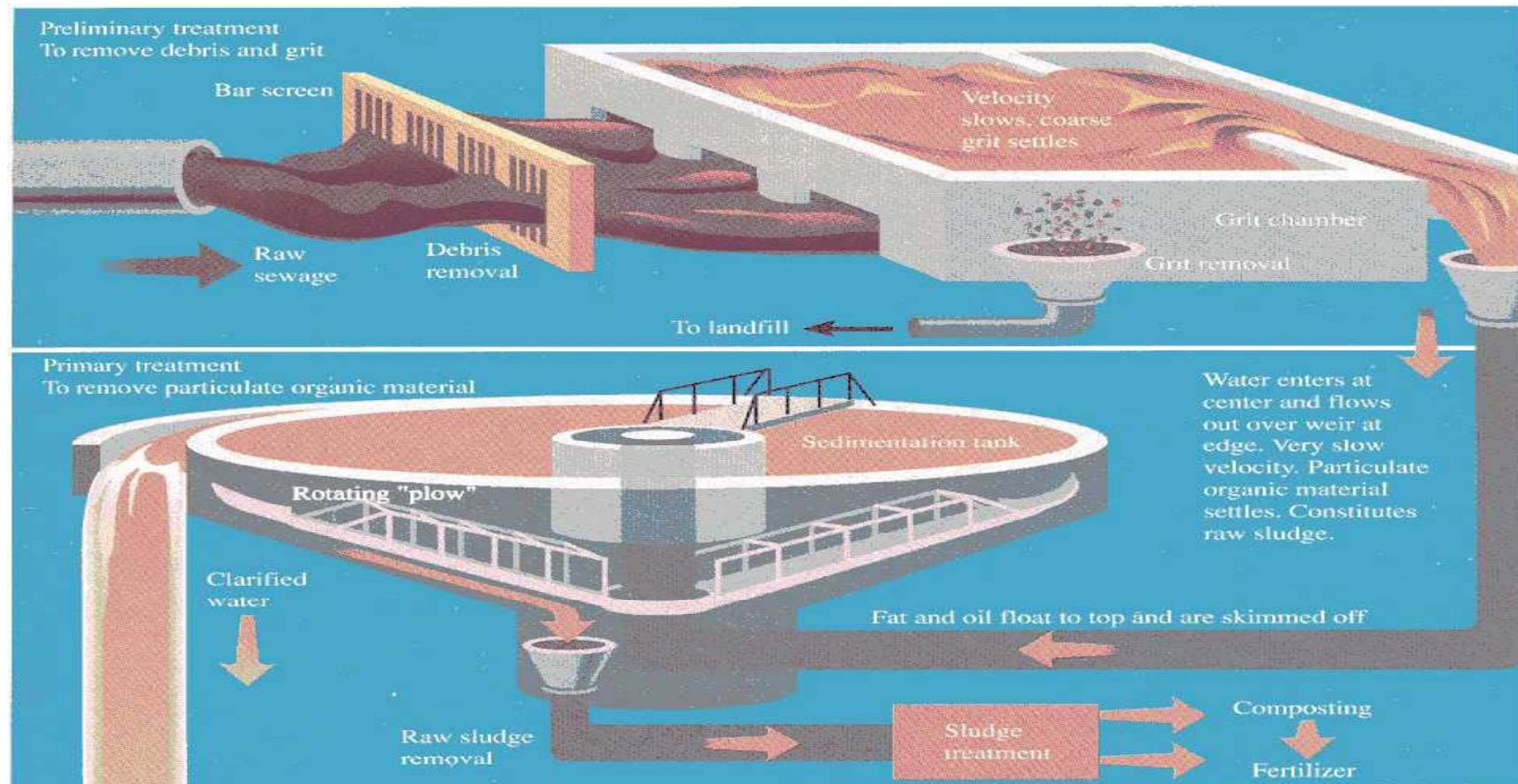
(b)

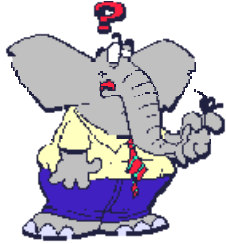
Primary (Treatment) sedimentation

Aim:

- **Removal of settable solids.**
- **Removal of 40% of BOD5:**

Primary (Treatment) sedimentation



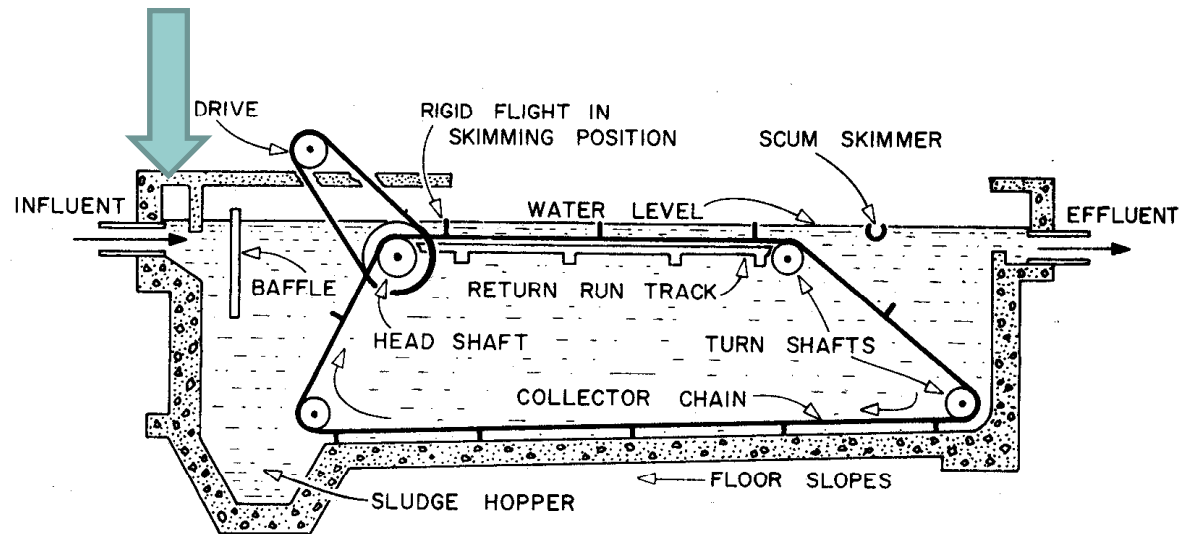


Proper Primary Treatment will
reduce reactor size
(construction cost)
and
Reduce power consumption
(operational cost)

Sedimentation tank consist of four zones:

1. Inlet zone:

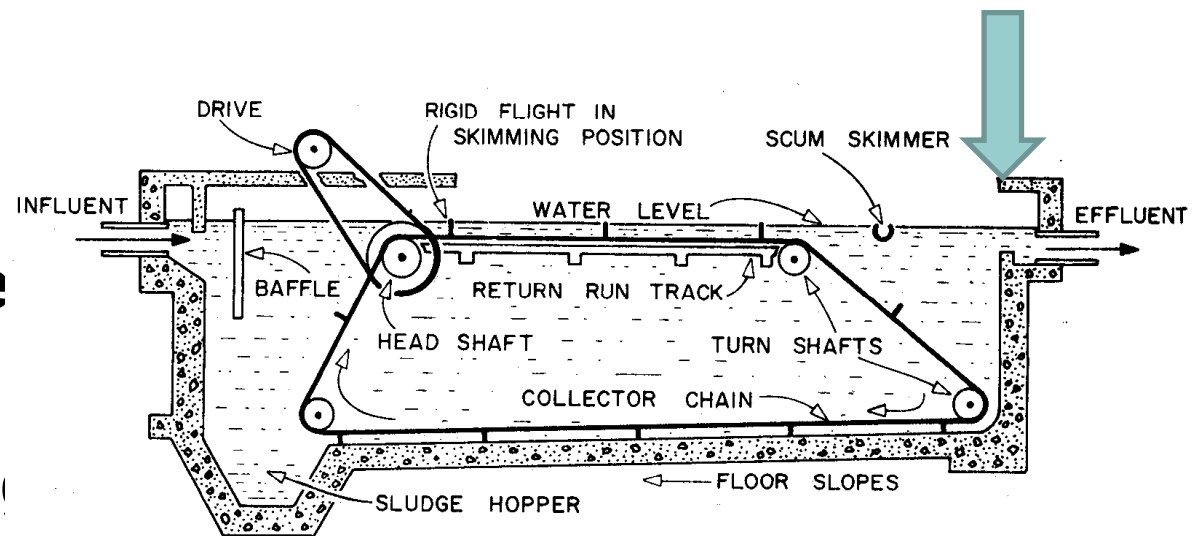
reduce the sewage velocity in a minimum space



2. Outlet weir :

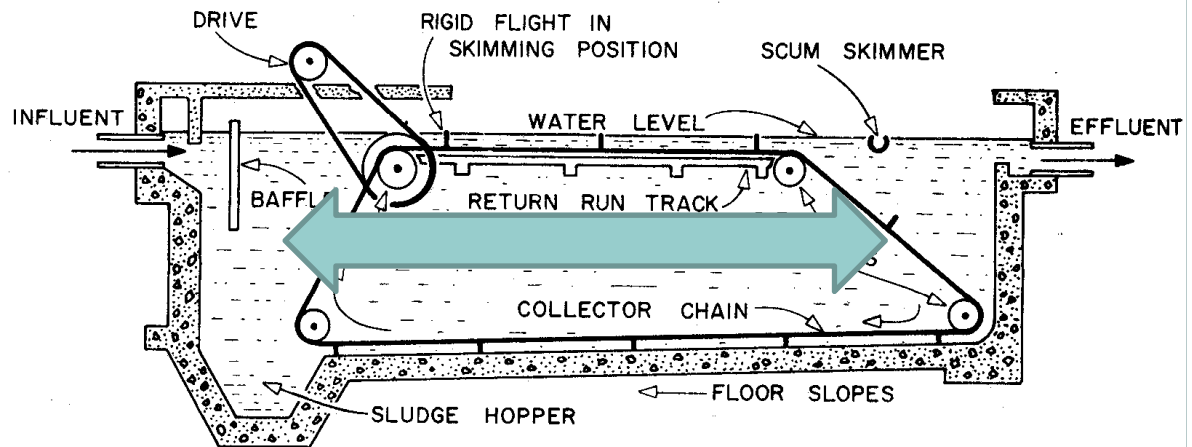
➤ **Collect and withdraw the settled sewage**

➤ **Prevent floating material to pass over outlet weir.**



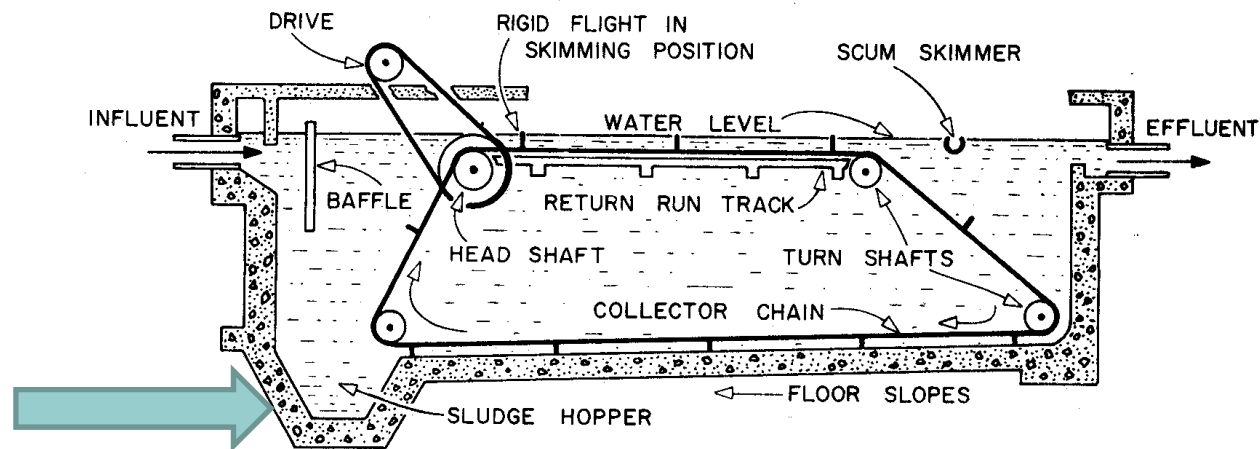
3. Settlement zone:

- ❖ **Area for settling**
- ❖ **Should free from:**
 - Short-circuiting
 - Stagnant areas

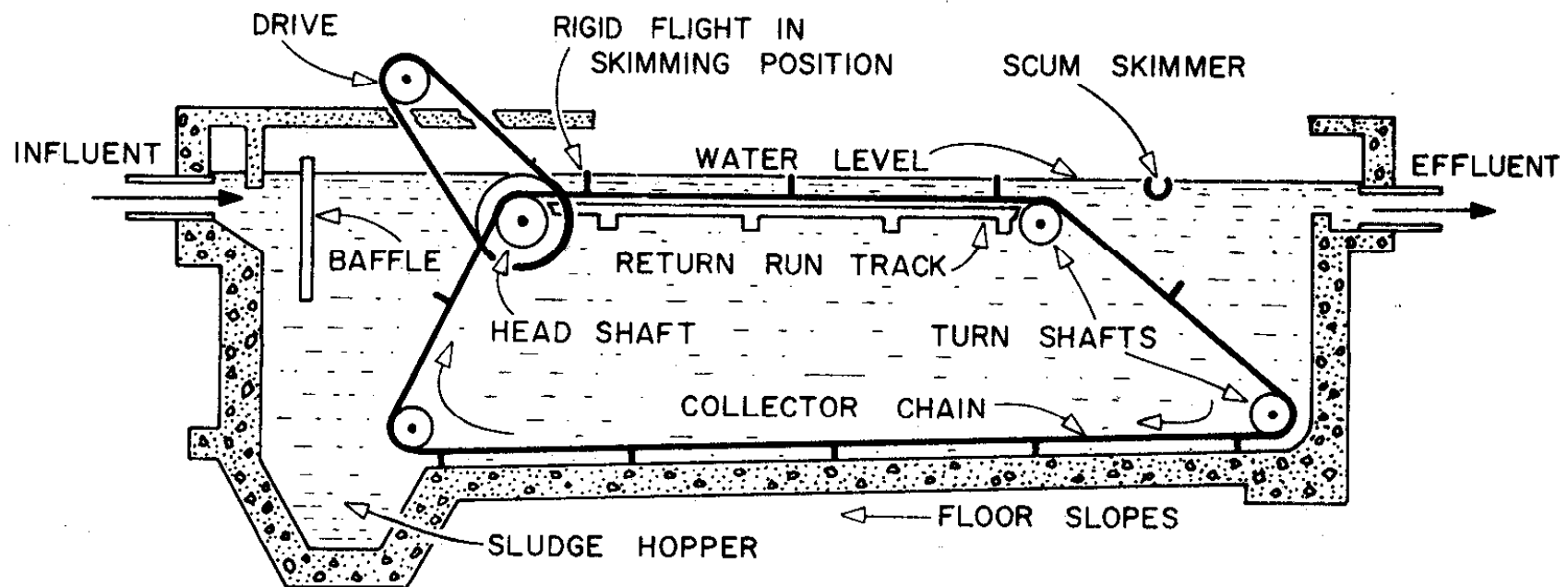


4.Storage , collection and withdraw of sediment solids (desludging):

- Manual
- mechanical



Primary Settling



ST type



1. Horizontal flow tanks:

Rectangular

Length / Breadth ratio 4:1

Horizontal flow tanks



ST type

2. Radial flow tanks:

- **circular with slope down floor.**
- **Inlet of the center and flows up and out**
- **Outlet is around the tank.**

2. Radial flow tanks:



ST type

3. Upward-flow tanks:

- **Circular or square.**
- **Inlet at tank center and flow initially downwards.**
- **Outlet is around the tank.**

Upward-flow tanks

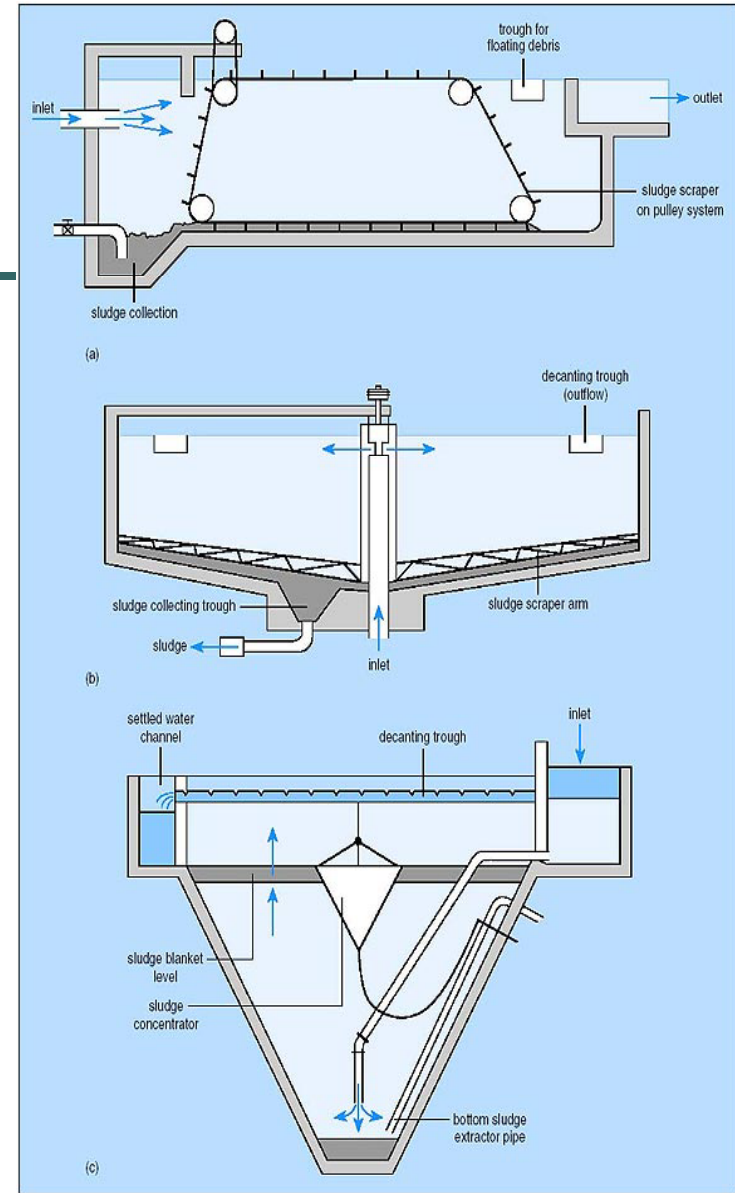


Typical sedimentation tanks:

(a) rectangular horizontal flow tank

(b) circular, radial-flow tank

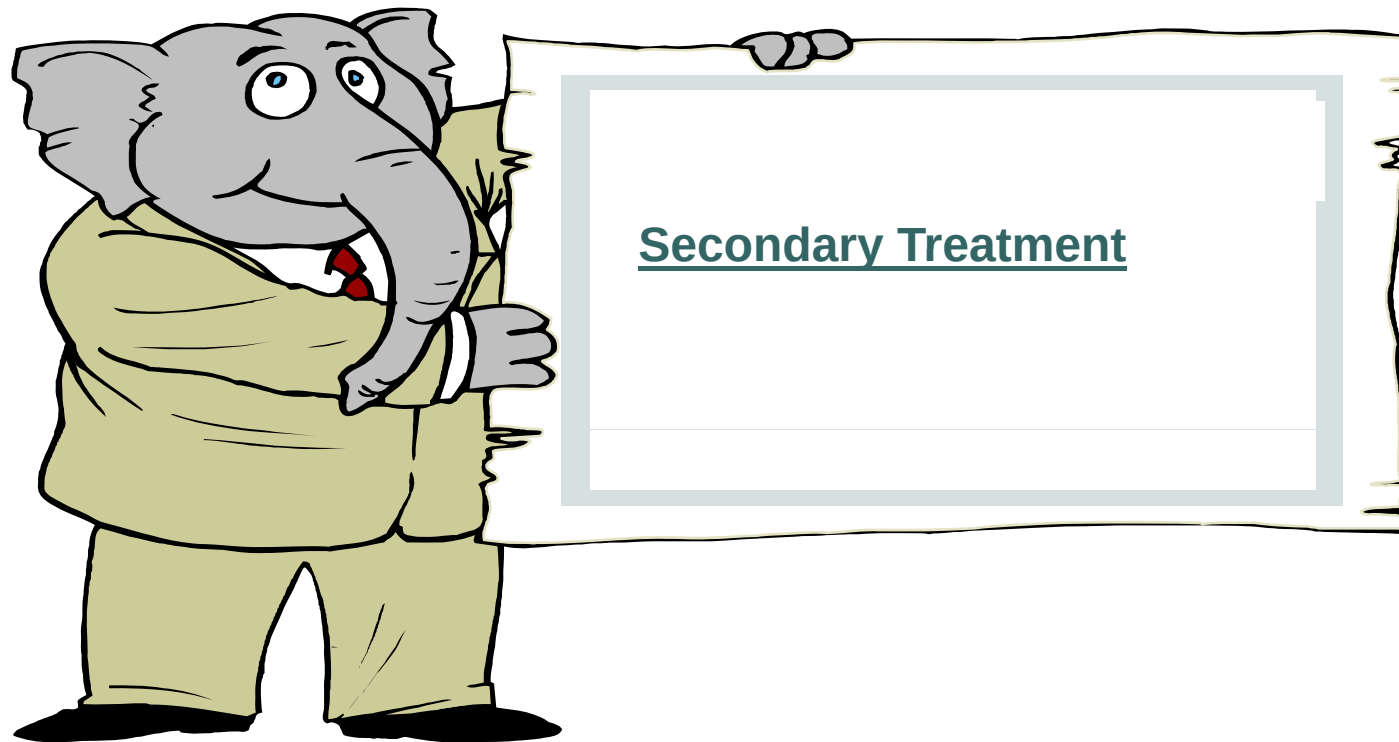
(c) hopper-bottomed, upward flow tank



Primary Settling Tank Design

- Size
 - rectangular: 3-24 m wide x 15-100 m long
 - circular: 3-90 m diameter
- Detention time: 1.5-2.5 hours
- Overflow rate: 25-60 m³/m²·day
- Typical removal efficiencies
 - solids: 50-60%
 - BOD₅: 30-35%

Secondary Treatment



Secondary Treatment

The main Aim of Conventional Secondary Treatment (Biological Unit) is to:

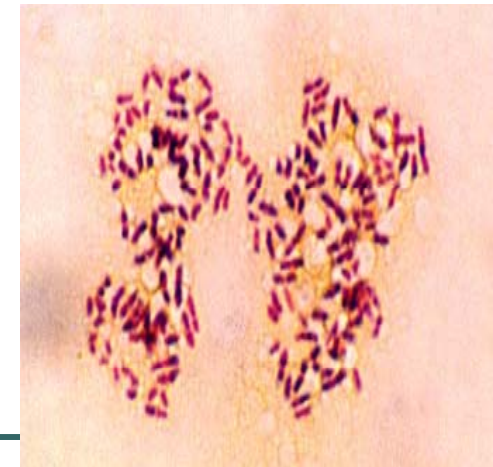
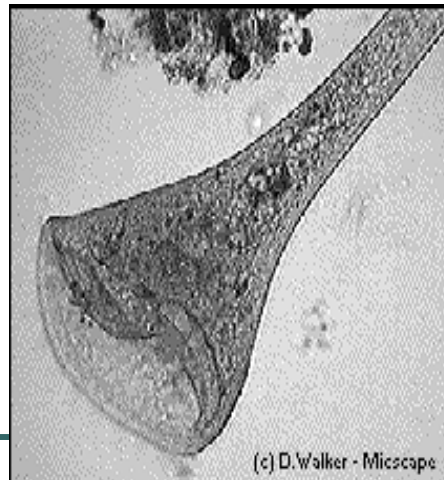
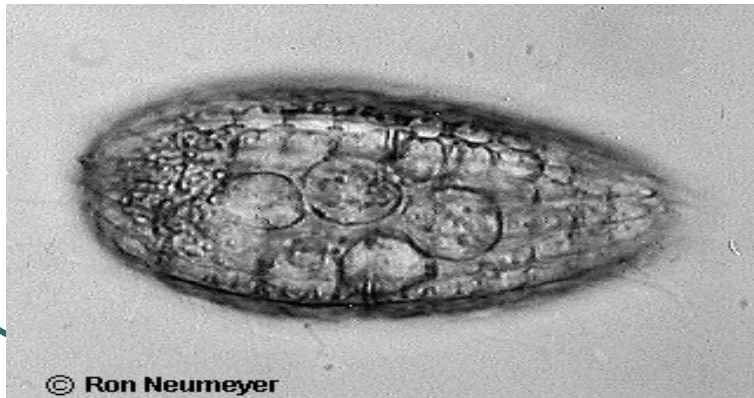
Reduce the Biological Oxygen Demand (BOD) through Aerobic Oxidation.

The process needs:

- **organisms**
- **oxygen**

How is this accomplished?

- Create a very rich environment for growth of a diverse microbial community



Secondary Treatment

- Provide BOD removal beyond what is achieved in primary treatment
 - removal of soluble BOD
 - additional removal of suspended solids
- Basic approach is to use *aerobic* biological degradation:



- Objective is to allow the BOD to be exerted in the treatment plant rather than in the stream

Basic Ingredients

- High density of microorganisms (keep organisms in system)
- Good contact between organisms and wastes (provide mixing)
- Provide high levels of oxygen (aeration)
- Favorable temperature, pH, nutrients (design and operation)
- No toxic chemicals present (control industrial inputs)

Process Classification based on form of microbial population:

- **fixed film processes**
(Microorganisms are attached)
- **dispersed growth processes**
(Microorganisms are immobilized)



Dispersed growth vs Fixed Growth

- Dispersed Growth – suspended organisms
 - Activated sludge
 - Oxidation ditches/ponds
 - Aerated lagoons, stabilization ponds
- Fixed Growth – attached organisms
 - Trickling filters
 - Rotating Biological Contactors (RBCs)

Fixed film processes:

- **“Fixed film processes represent the oldest form of wastewater treatment system and includes”:**

cont==Fixed film processes:

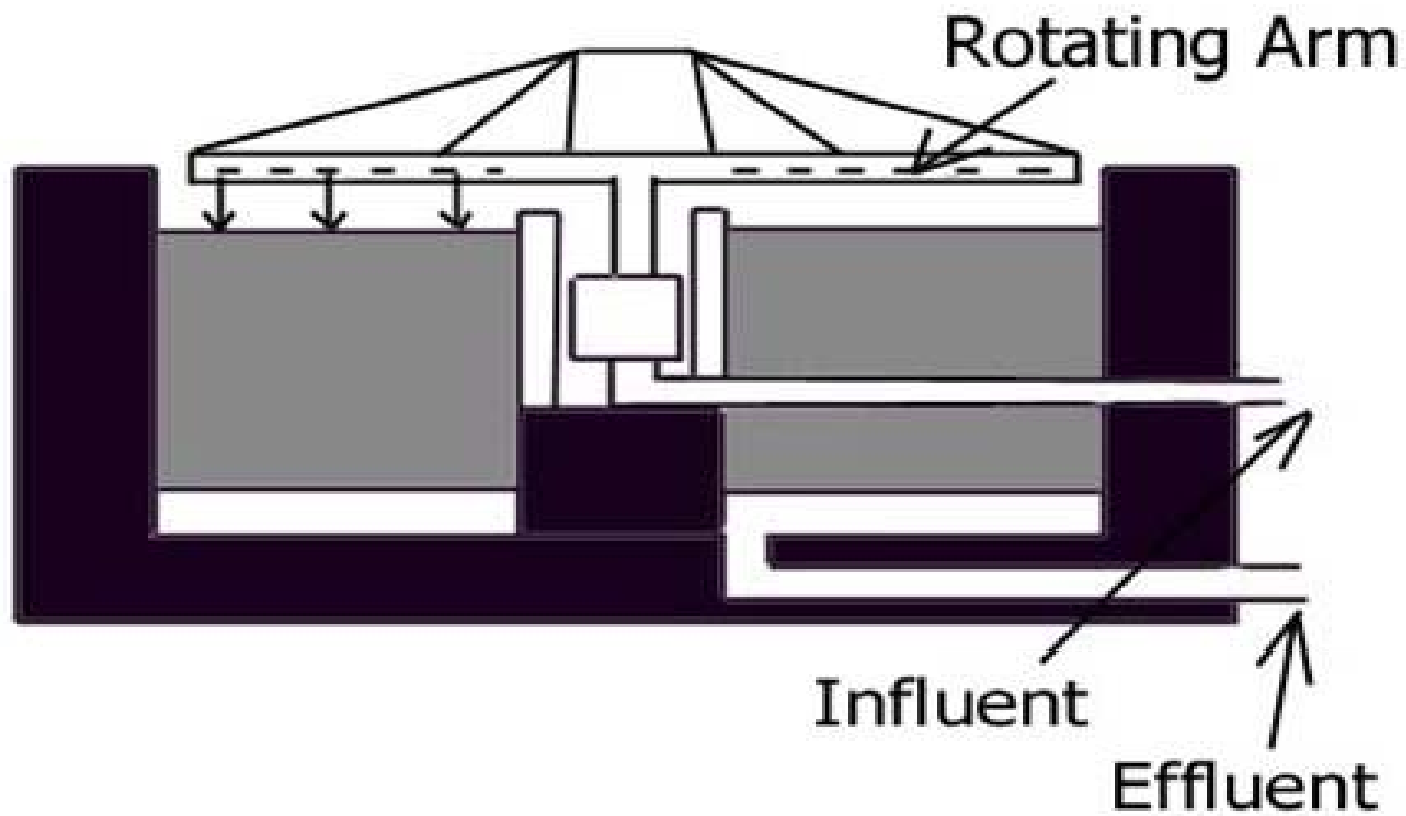
- **Land application of sewage**
- **Trickling or Percolating filter**
- **Rotating Biological Contractor (RRC)**

Trickling filter

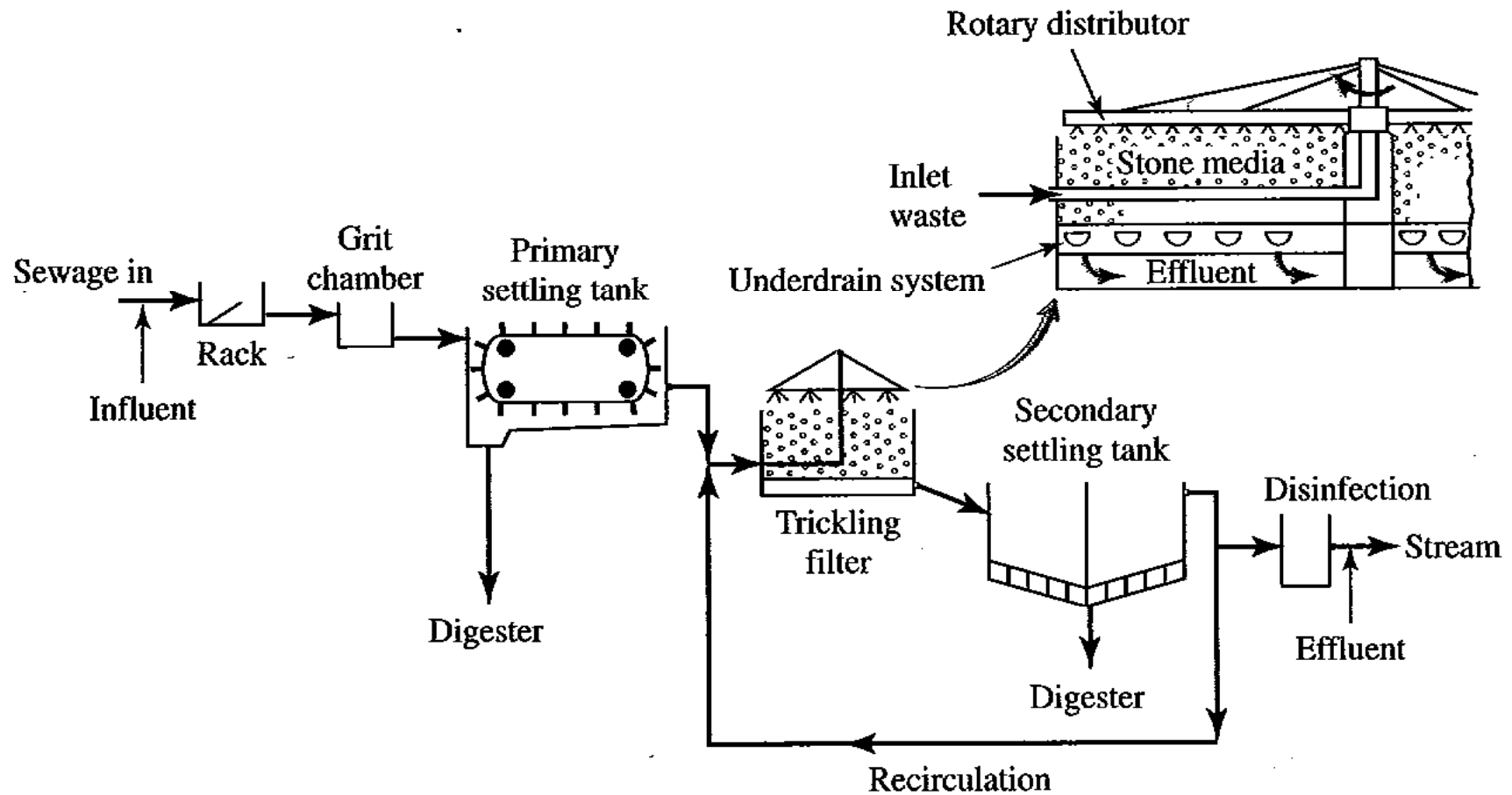
Trickling filter

**“Bio-filter, bacteria bed
or
percolating filter”**

Trickling Filters

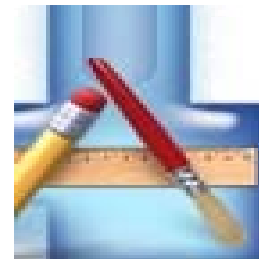


Trickling Filter Plant Layout

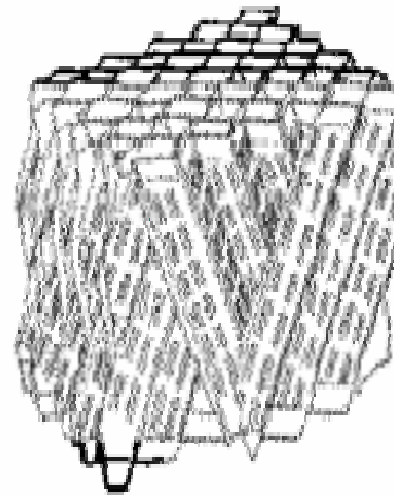


Trickling filter:

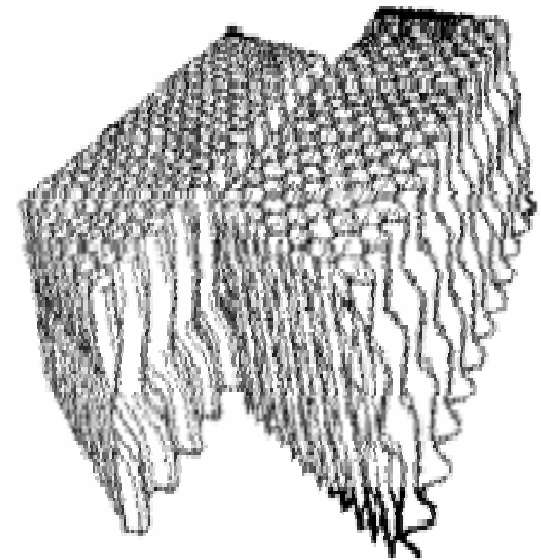
- **Form:** Rectangular or Circular Plan
- **Inside:** A permeable Media (crushed rock or blast-furnace slag of diameter 25-100mm)
- **Depth:** 3.0m to 12m (depends of the filter media)



Trickling Filt



Cross-flow

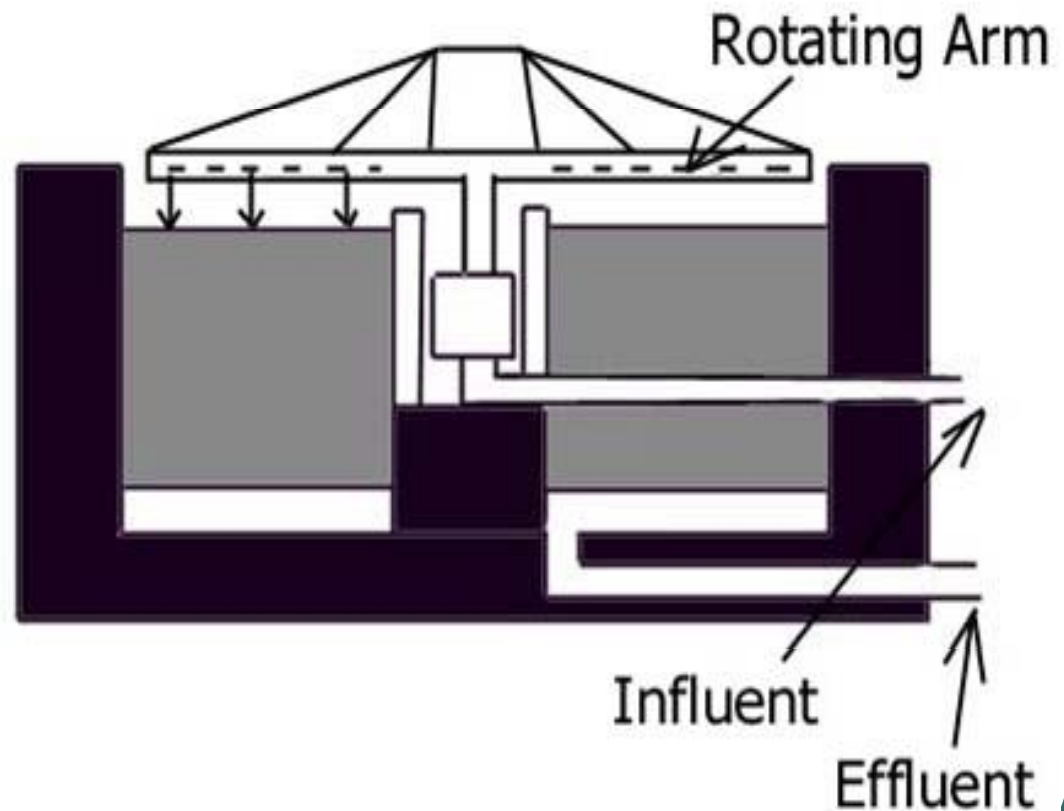


Tubular

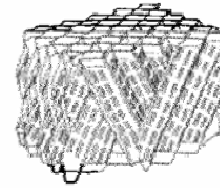


Trickling filter:

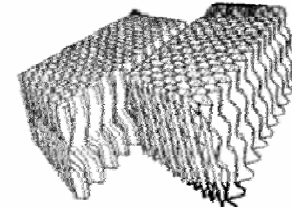
- Wastewater distributed mechanically over the media
- Effluent collected at the filter base



Trickling Filters



Cross-flow



Tubular



Treatment Process:

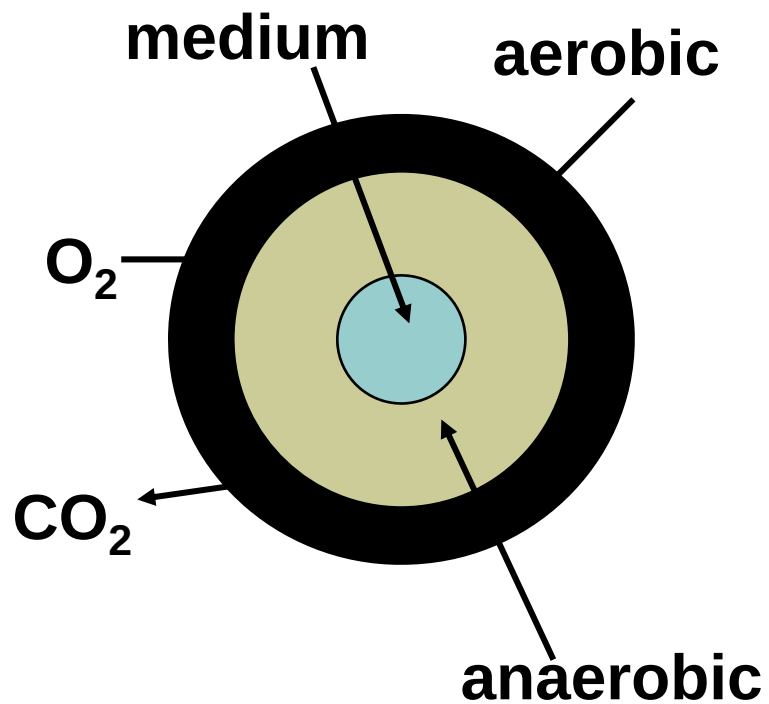
- **Influent entering at the center of the bed and passing into radial distributor arms above the bed surface**

cont== **Treatment Process**

- **A microbial film develops over the surface of the media and this is responsible for removal of BOD during passage of sewage through the bed.**

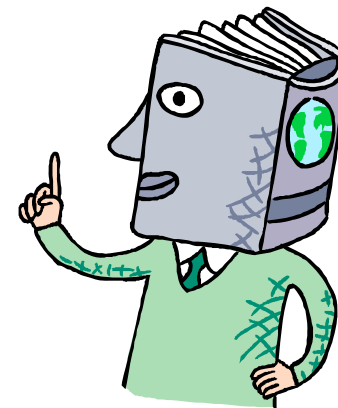


cont== Treatment Process



cont== **Treatment Process**

- **The distribution of air through a filter occurs by circulation through void spaces in the filter media.**



cont== Treatment Process

- **Filtration, in the traditional sense of straining out of large particles, does not occur and the process is solely a biological one**

The efficiency of the system depends upon:

- Distribution of settled sewage over the surface of the filter**
- Circulation of air throughout the filter media**



Factors Effecting Filters Loading Rates:

- 1. Effluent quality required**
- 2. Media employed**
- 3. Type of wastewater (Influent quality)**

PONDED Problem:

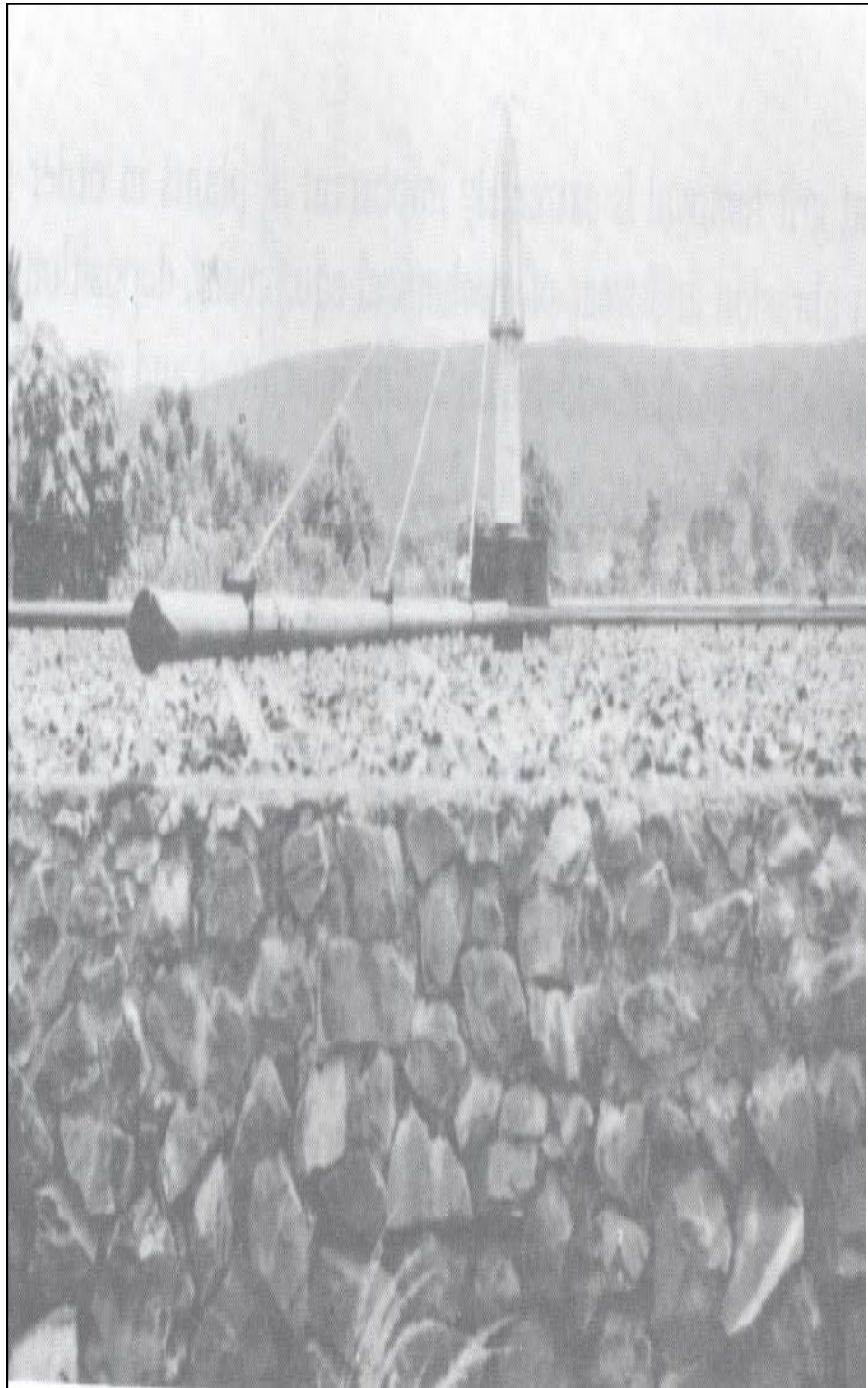
- **Over Loading cause excessive growth of the slime layer resulting in the voids between the media and it becoming blocked.**



PONDED Problem:

**This prevents sewage
percolating through the filter
and thus it collects on the
surface of the media**

‘Ponded Cause’



Sloughing off':

- **In a well operated filter there is a continual cycle of film growth followed by death and detachment from the media. This is known as 'Sloughing Off'.**

Cont==Sloughing off':

- **The resultant sludge is carried away with the filter effluent. Periodic sloughing off the slime layer is a feature of filter operation (change of seasons). During this period treatment will be severely reduced.**

Trickling Filters

- Not a true filtering or sieving process
- Material only provides surface on which bacteria to grow
- Can use plastic media
 - lighter - can get deeper beds (up to 12 m)
 - reduced space requirement
 - larger surface area for growth
 - greater void ratios (better air flow)
 - less prone to plugging by accumulating slime



Advantage of trickling filters:

- **Simple technique to design and operate**
- **Have very low running costs**
- **Tolerate shock and toxic loads (short contact time)**



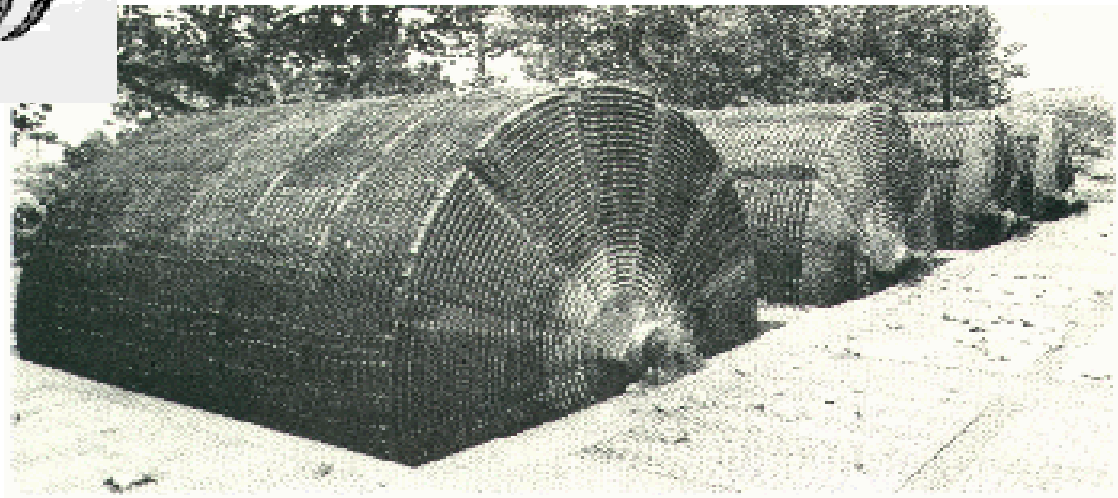
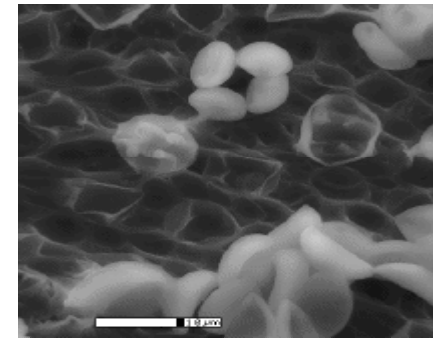
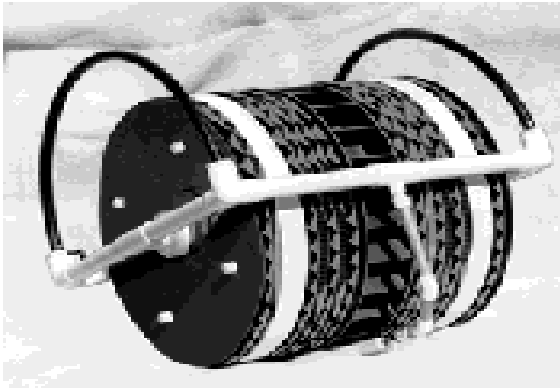
Disadvantages:

- **Land requirements high**
- **Provide a limited treatment efficiency**
- **Associated with odorous and fly nuisance (In hot countries)**

Rotating biological contactor (RBC) (Biodisc):

- **A rotating disc (bed) has attached bacteria**
- **The disc immersed in wastewater tank**

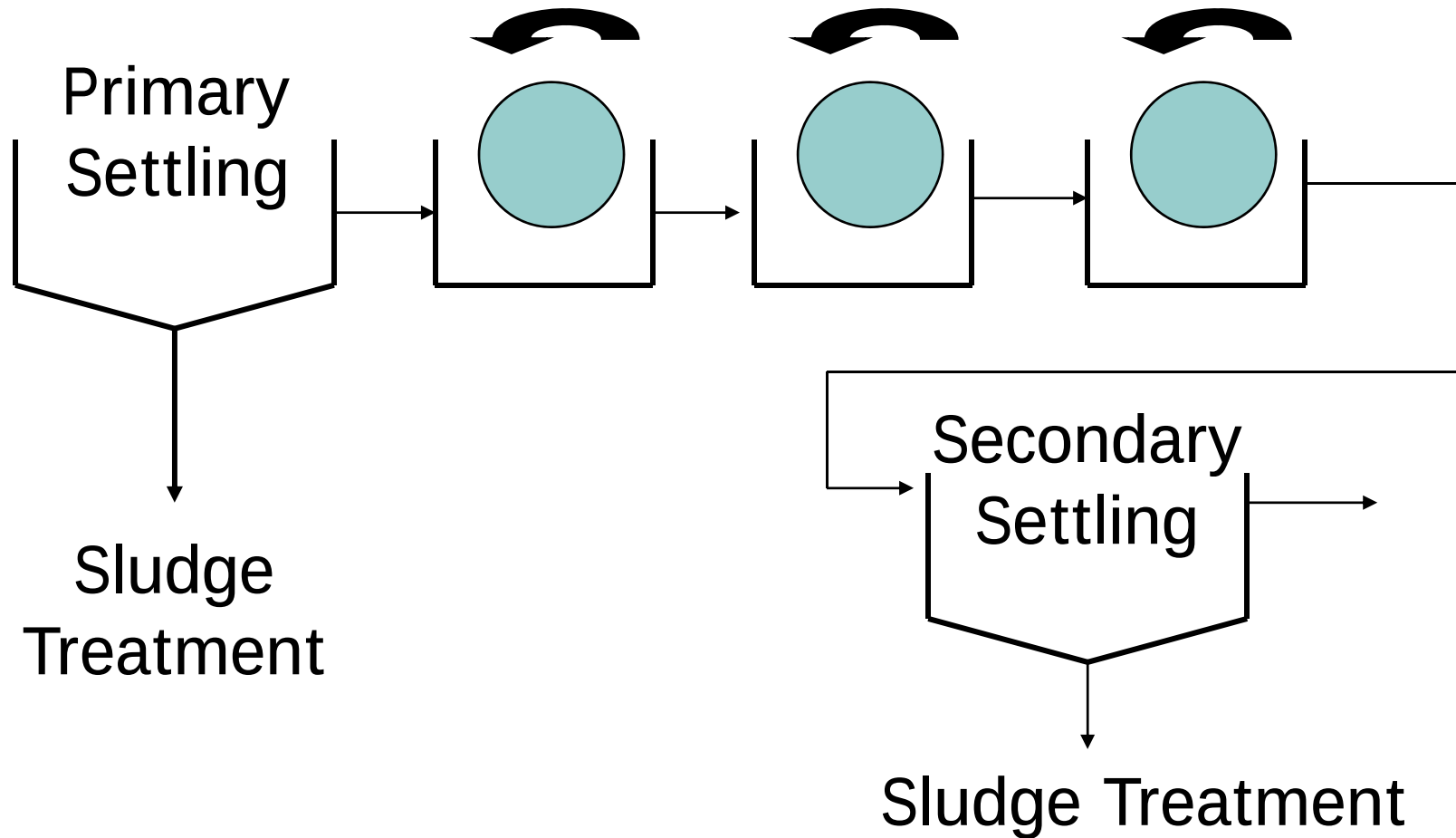
Rotating Biological Contactors



Rotating Biological Contactors

- Called RBCs
- Consists of series of closely spaced discs mounted on a horizontal shaft and rotated while ~40% of each disc is submerged in wastewater
- Discs: light-weight plastic
- Slime is 1-3 mm in thickness on disc

Rotating Biological Contactors



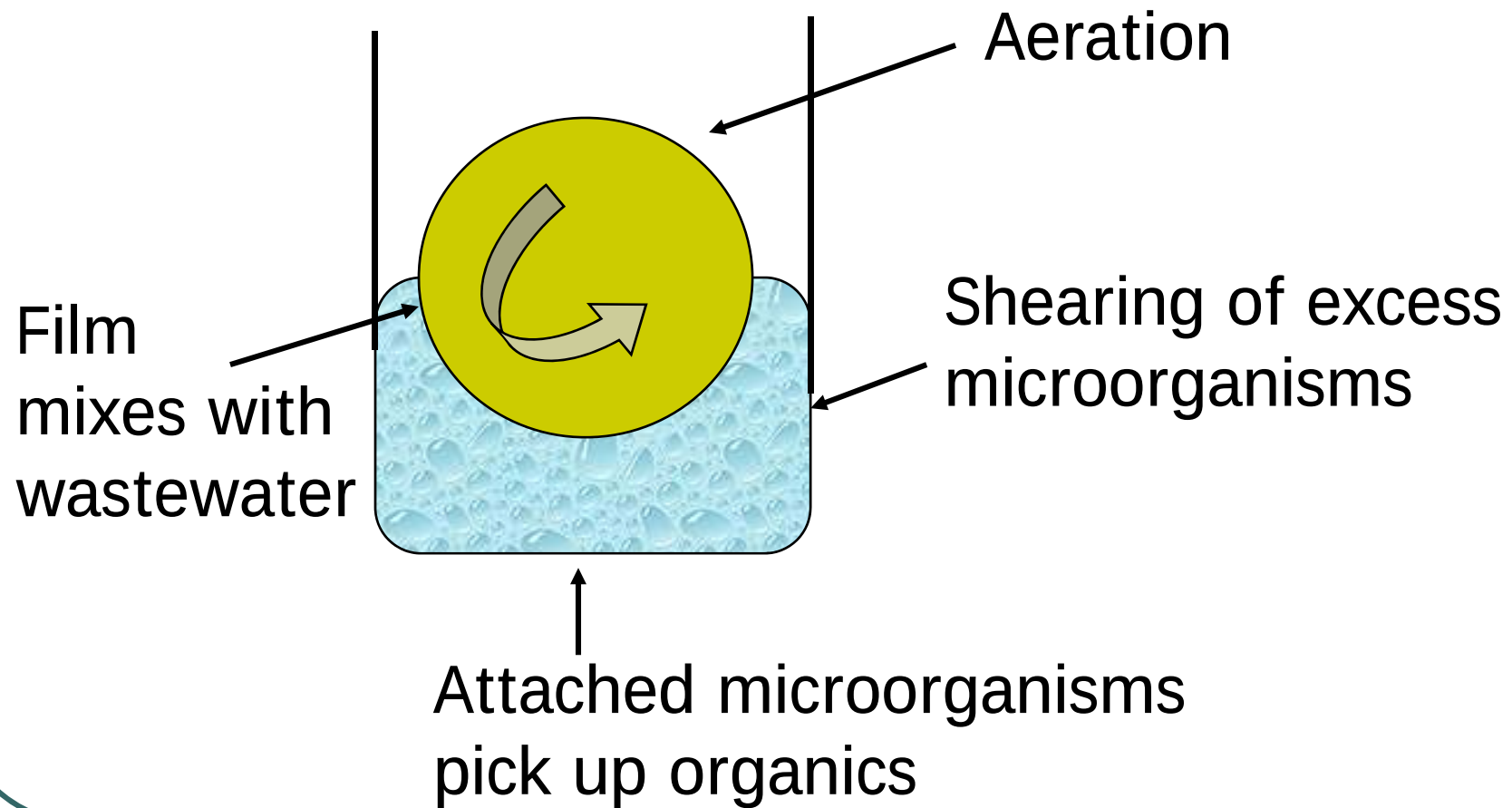
Cont==Rotating biological contactor (RBC) ,(Biodisc):

- **Rotation of disk will permitting the aeration as disc expose to the atmosphere**

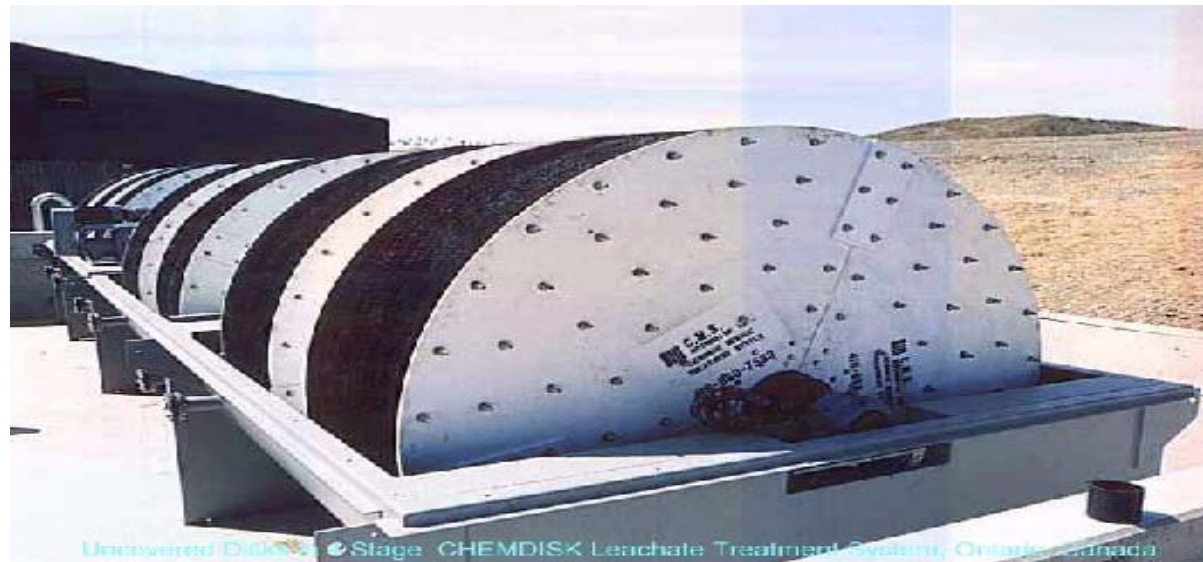
Cont==Rotating biological contactor (RBC) ,(Biodisc):

- **A microbial community will be developed at the disk (from of biofilm = 3mm) which is responsible for BOD removal**

Rotating biological contactor (RBC), (Biodisc)



Rotating Biological Contactors



Advantages:

- **Ease to operate**
- **Low land requirement**
- **Application in rural areas**
- **Can be marketed as package**
- **Reduced power and maintenance cost**
- **Elimination of pounding and clogging**

Disadvantages:

- **New process**
- **Breaking and craking of discs**
- **Shaft bearing failure**
- **Rain and wind protection faultily**

Rotating Biological Contactors



Dispersed Growth Processes

Activated Sludge Process:

The process is a suspended growth system comprising a mass of microorganisms constantly supplied with organic matter and oxygen.

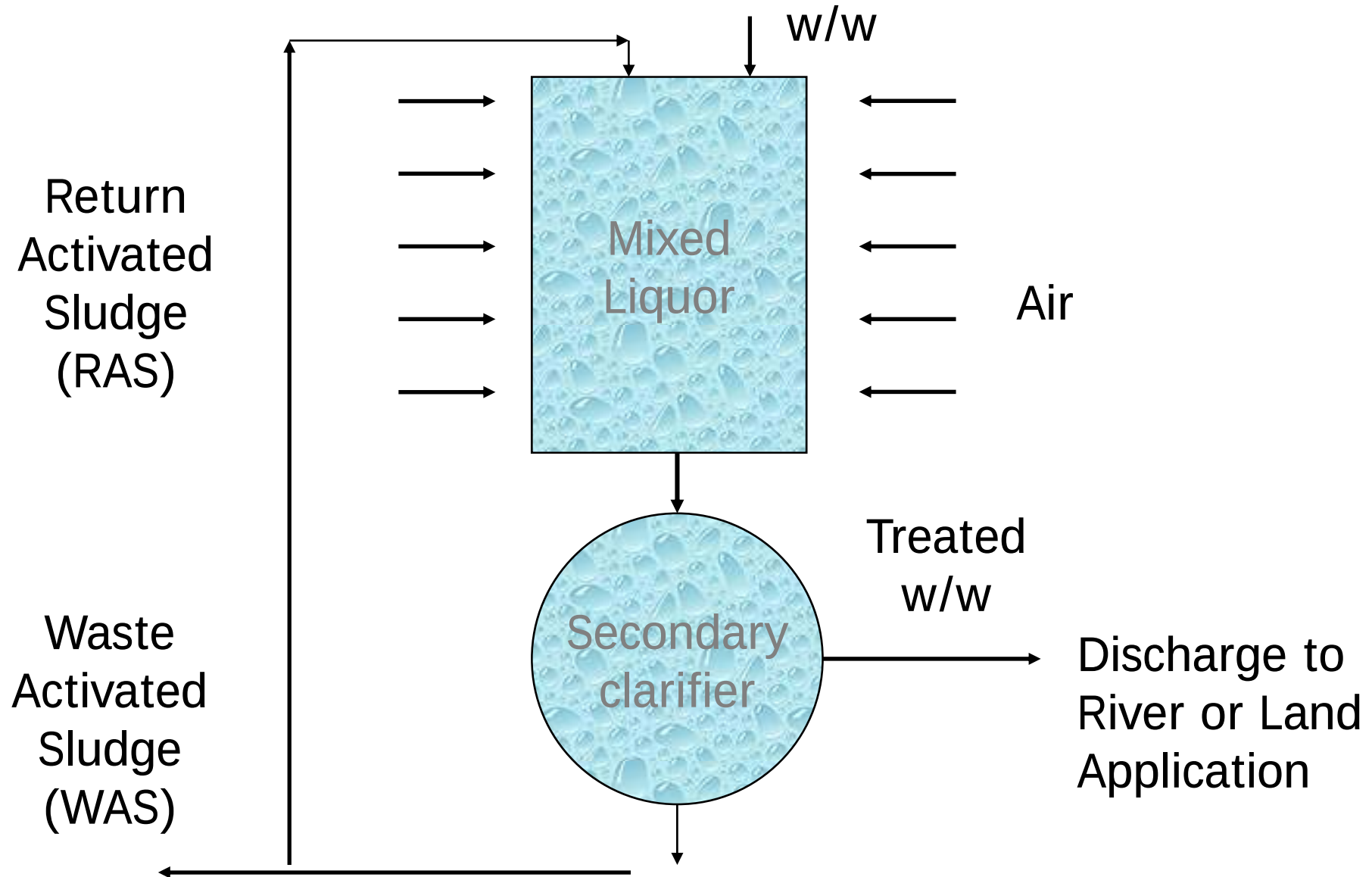
Cont==Activated Sludge Process:

The microorganisms grow in flocs, and these flocs are responsible for the transformation of the organic material into new bacteria, carbon dioxide and water.

Cont==**Activated Sludge Process:**

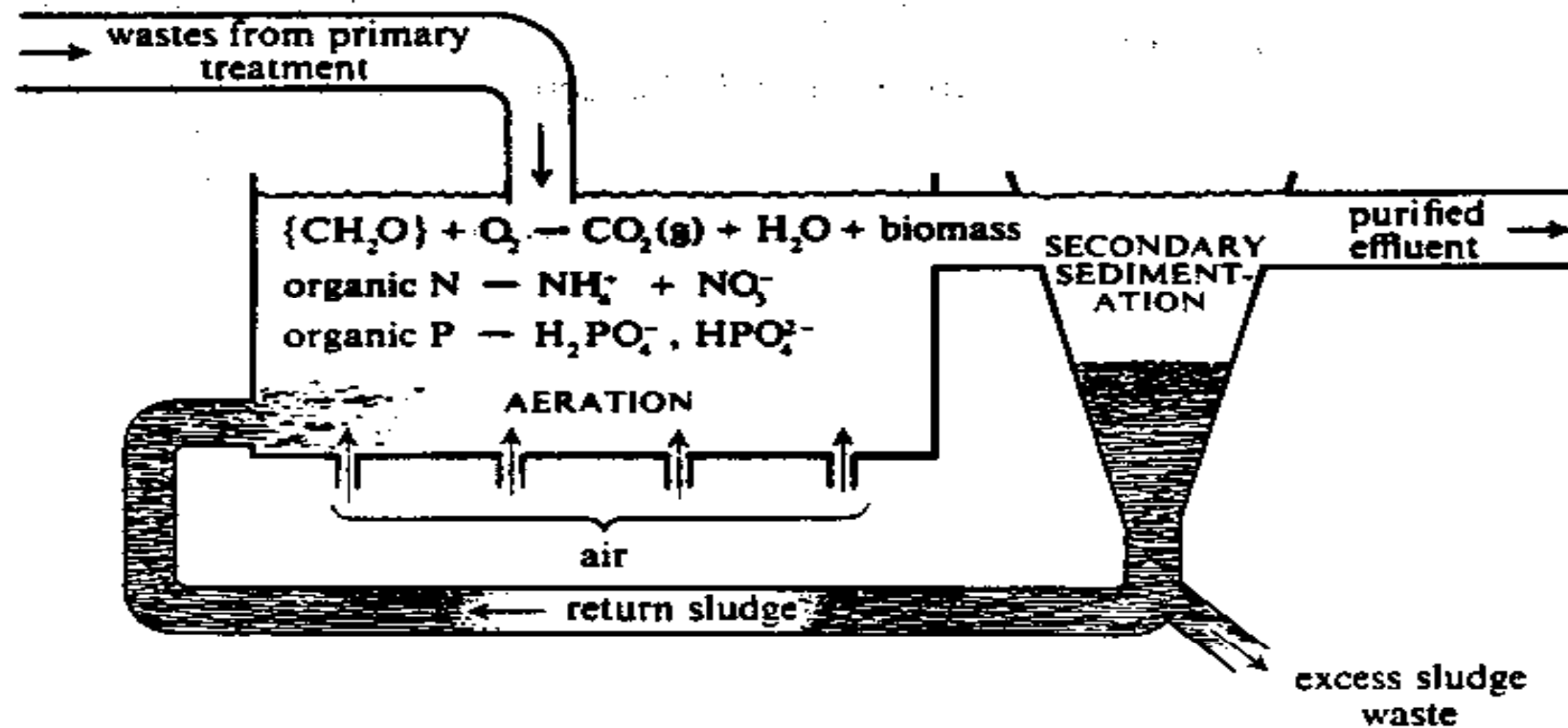
- Process in which a mixture of wastewater and microorganisms (biological sludge) is agitated and aerated
- Leads to oxidation of dissolved organics
- After oxidation, separate sludge from wastewater

Activated Sludge



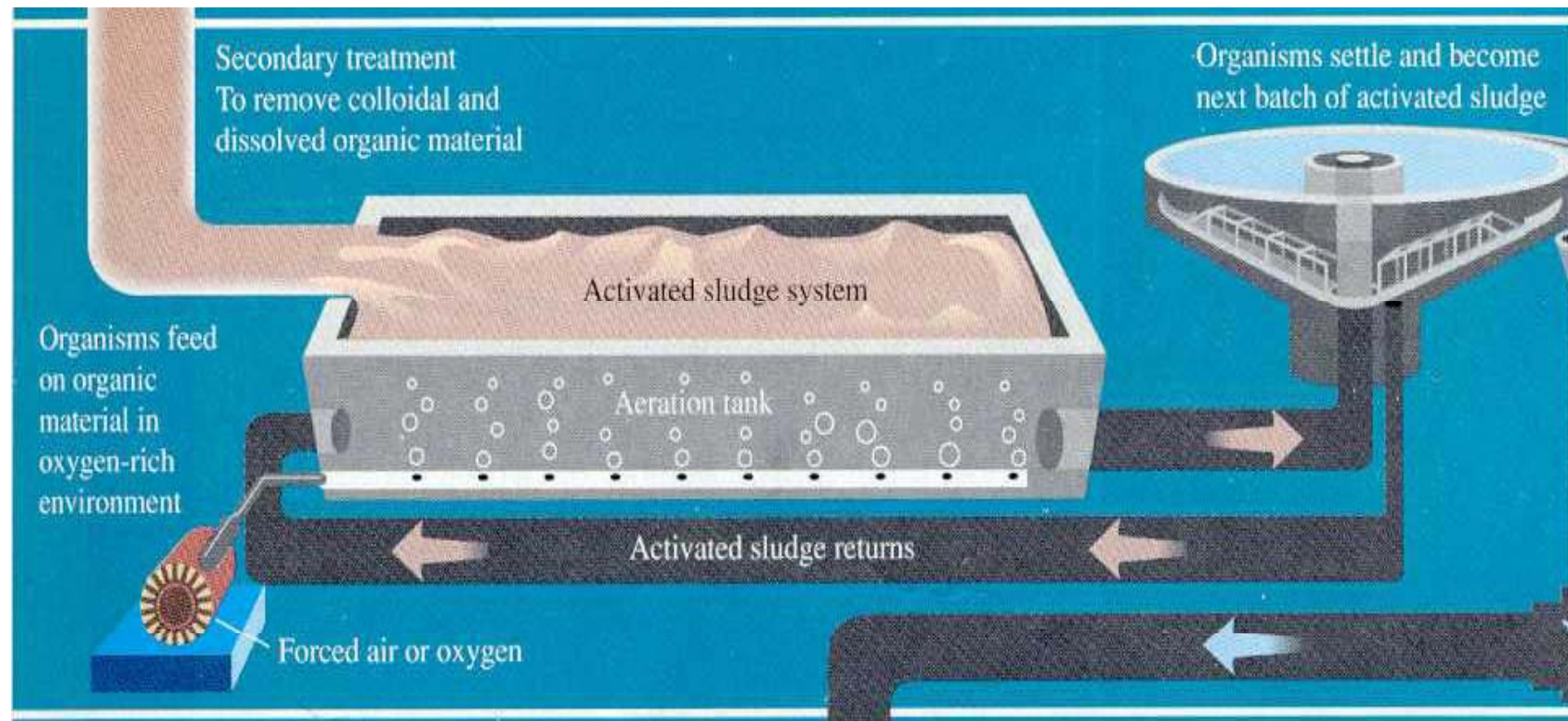
Dispersed Growth Processes

Activated Sludge Process:



Dispersed Growth Processes

Activated Sludge Process:



Activated Sludge Process



Activated Sludge Process

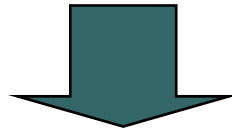
Organic Matter (BOD)

+

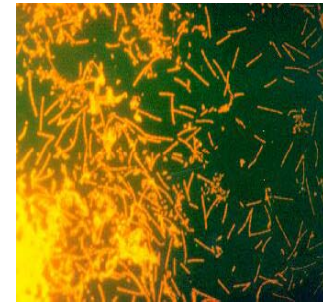
Oxygen

+

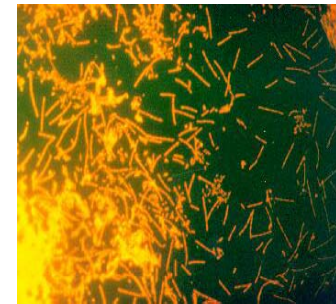
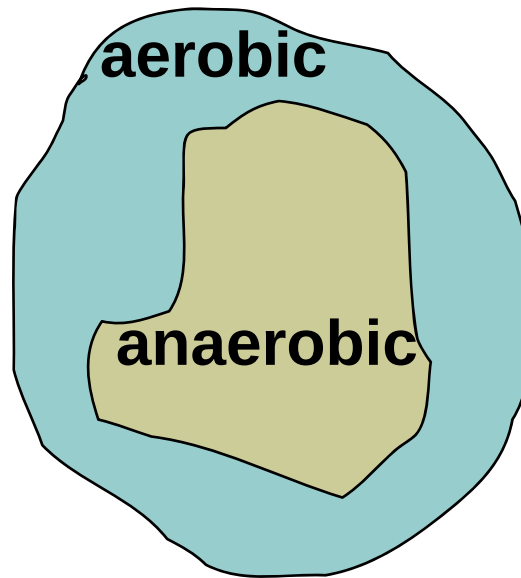
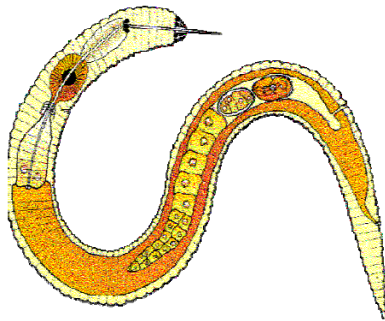
Microorganism



Growth (Flocs)



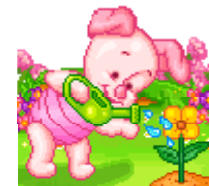
Sludge floc



Cont==Activated Sludge Process

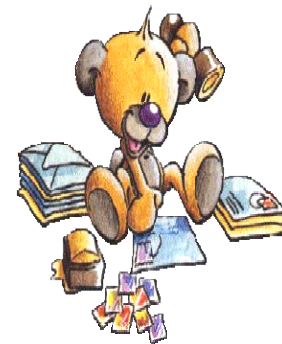
2.Flocs washing out to the secondary sedimentation tank

3.Settling of flocs (SLUDGE)

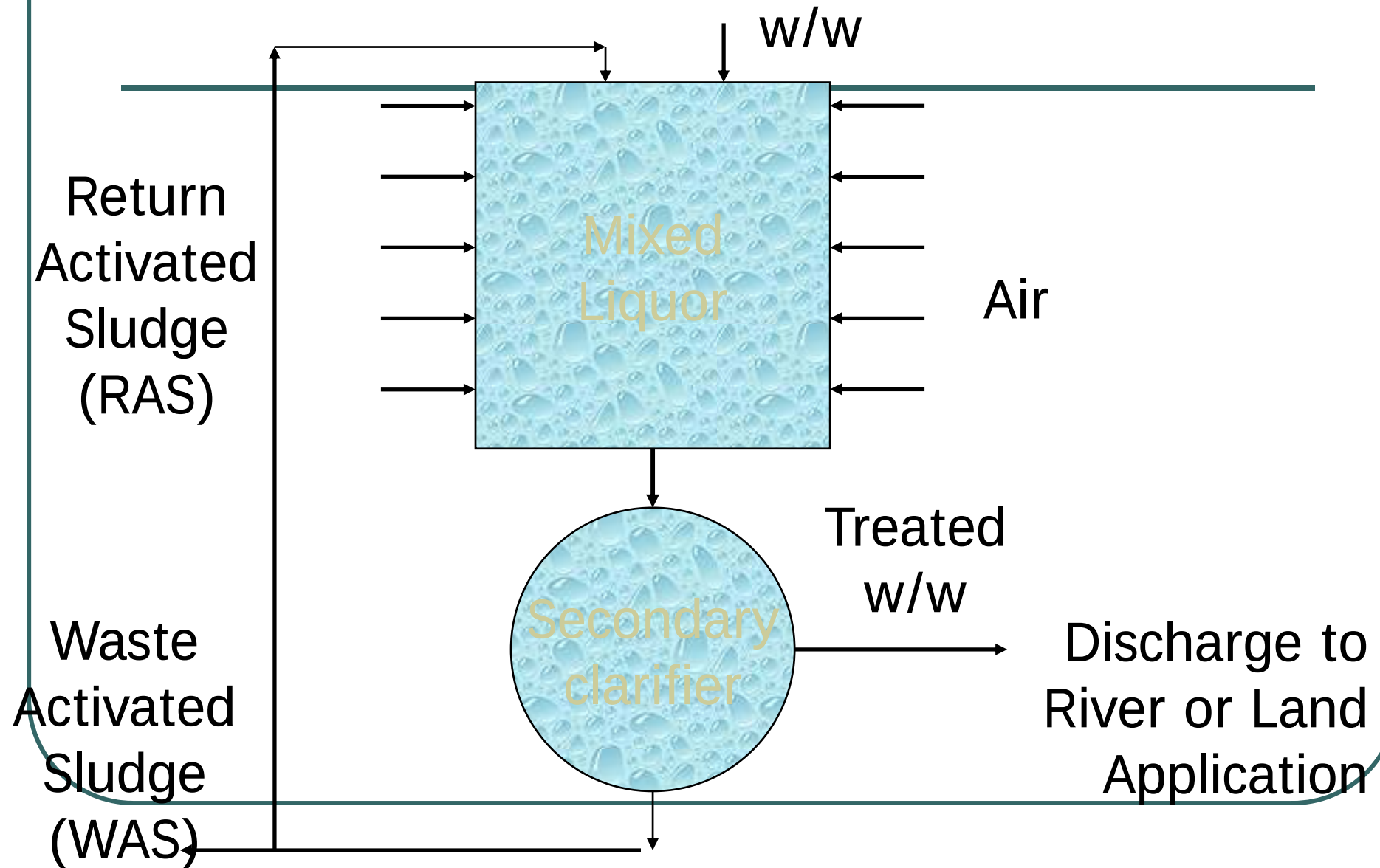


Cont==Activated Sludge Process

- 4. A fraction of settled sludge recycled back to the aeration tank (Returned Sludge)**
- 5. Remainder sludge will be wasted**

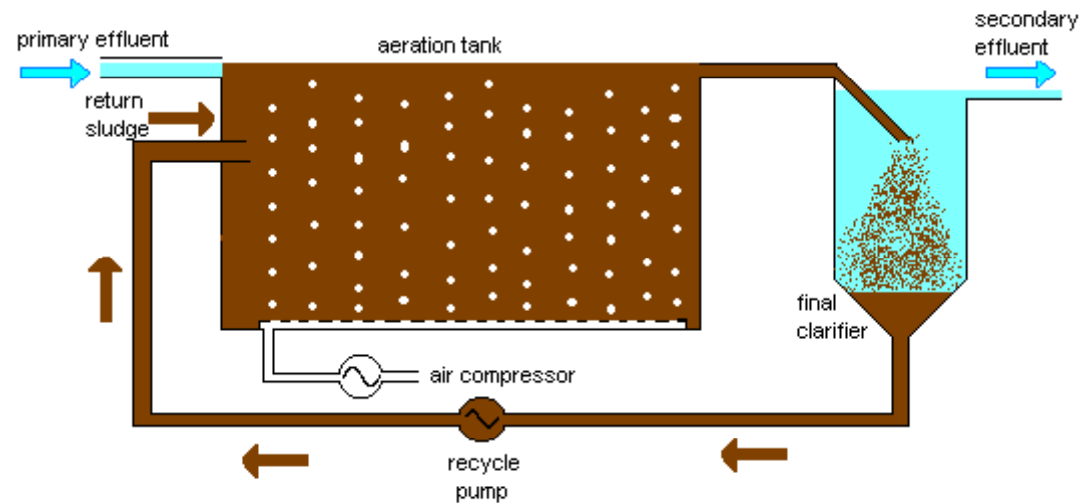


Activated Sludge

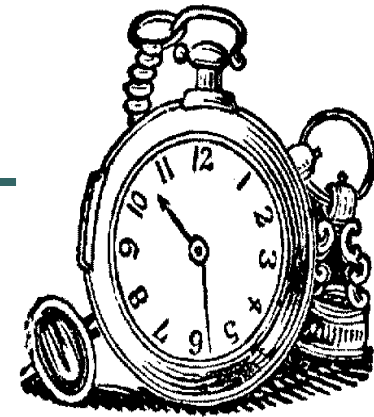


Activated Sludge Process

Activated Sludge Process



Sludge Age



The average time, which

A microorganism (fraction of the solids, which are wasted) will spend in the reactor is known sludge age

Sludge Age



- and is defined as:

$$\text{Sludge age (d)} = \frac{\text{Total solids in reactor (Kg)}}{\text{Total solids wasted (Kg/d)}}$$

Variation principals of activated sludge process

1. **The method of oxygen supply**
2. **Aeration tank configuration**
3. **Loading rate**



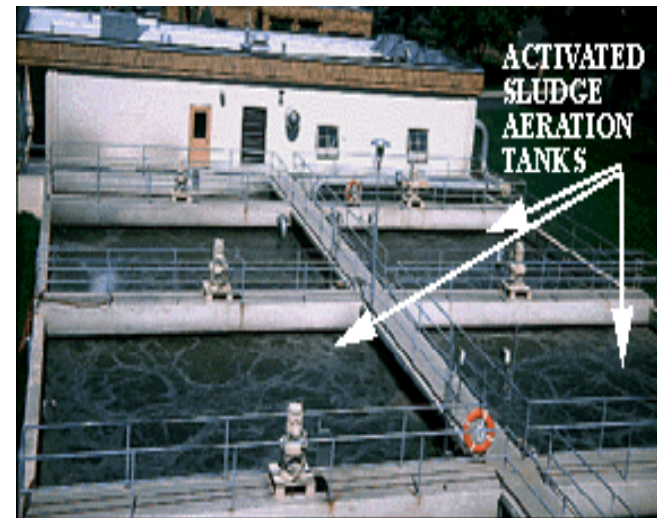
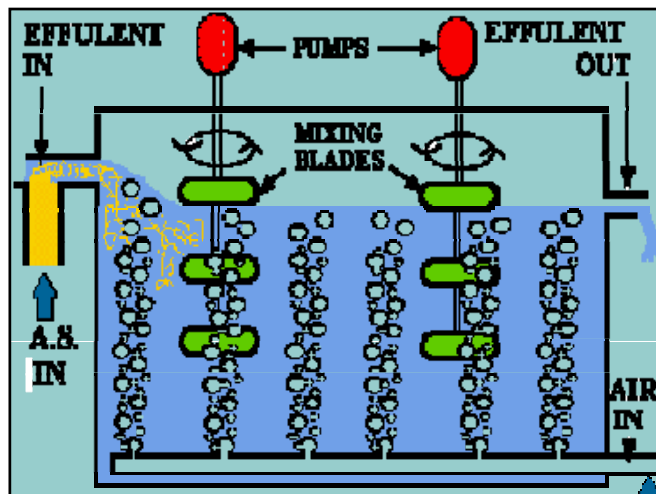
Methods of oxygen supply for activated sludge:

1. **Mechanical surface aeration.**
2. **Diffused air aeration.**

Methods of oxygen supply for activated sludge:

Diffused air aeration.

Mechanical surface aeration.



Mechanical surface aeration:

1. Transfer O₂ from atmosphere to the **MIXED LIQUOR** by agitation of the liquid surface



cont==Mechanical surface aeration:

2. **Agitation helps to keep the sludge (flocs) in suspension and prevents it settling:**
 - ❖ **Vertical A**
 - ❖ **Horizontal A**

Cont==Mechanical surface aeration:

The immersion depth of the aerator is very important and affect the oxygenation capacity



❖ Vertical A



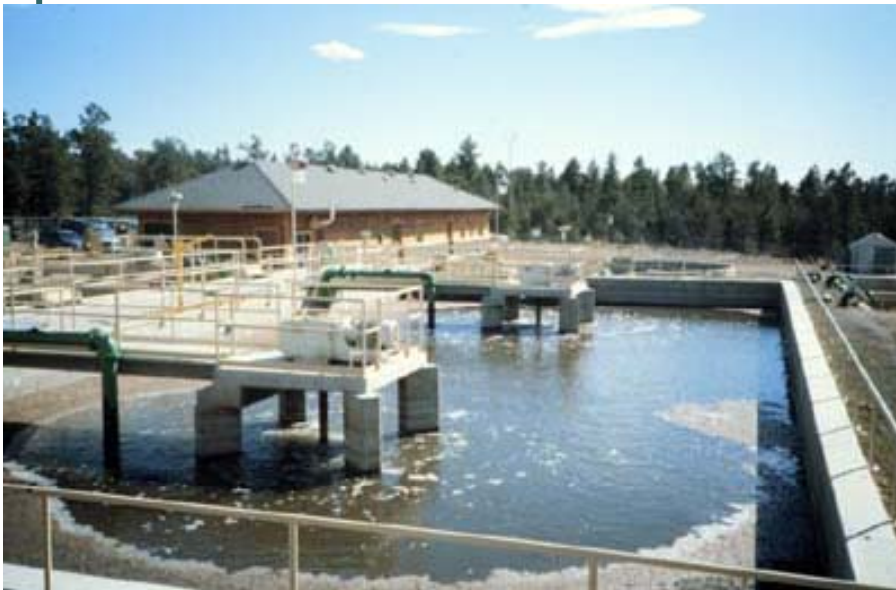
Mechanical Aerators



Fixed aerator



Floating aerator



Turbine aerator



Diffused air aeration:

- 1. Located at the floor of the reactor and release air stream into mixed liquor in form of bubbles**
- 2. As bubbles rise to tank surface, this will keep the mixed liquor solids in suspension form**

Diffused air aeration:



Ceramic Disc Air Diffuser



Diffusers



Type of air diffusers



1. Fine bubble diffused air:

- **Tiles made of ceramic**
- **Bubbles diameter up to 2mm**
- **Provide large air/liquid interface with high oxygen transfer efficiency**

Blockage problem



**attached biomass
particles in air**

2. Coarse bubble diffused air

- **Bubbles diameter in range 3-5 mm**
- **Reduced oxygen transfer**

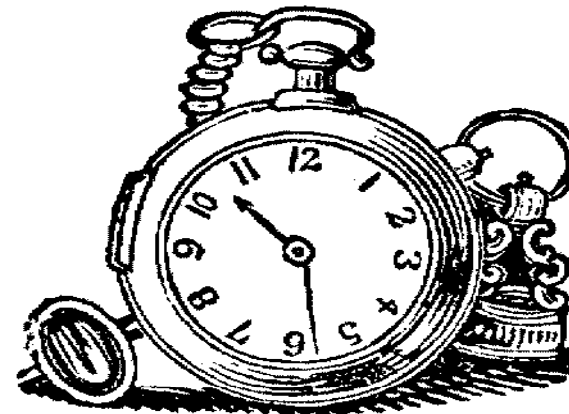
Loading rate

The parameter describe the rate of applying of sewage in the AS reactor

- 1. Volumetric loading**
- 2. Organic loading**
- 3. Food: microorganism ratio (f/m-ratio)**
- 4. Floc loading**

1.Volumetric loading:

Determined the time ,which the sewage will undergo aeration in reactor



Retention Time (d) [Hydraulic RT]

= Reactor volume (m³)/total daily flow (m³/d)

In cause of absence of sludge Recycle

RT= sludge age

➤ **Contact time (6-10)hs**

➤ **Aeration Tank magnitude**

2.Organic loading:

**The amount of organic (BOD) applied per
unit volume of aeration tank**

2.Organic loading:

$$\text{Organic Loading (Kg BOD/m}^3\text{.d)} = \frac{\{\text{influent flow (m}^3\text{/d)} * \text{BOD (kg/m}^3)\}}{\text{Reactor volume(m}^3\text{)}}$$

3. Food-Microorganism Ratio (F/M-Ratio):

- **Sludge loading**

F/M-Ratio=

$\frac{\text{BOD of sewage (Kg/m}^3\text{)} * \text{influent flow (m}^3\text{/d)}}{\text{Reactor volume (m}^3\text{)} * \text{Reactor Sludge (Kg/m}^3\text{)}}$

F/M Parameter

- Low F/M (low rate of wasting)

 - starved organisms
 - more complete degradation
 - larger, more costly aeration tanks
 - more O₂ required
 - higher power costs (to supply O₂)
 - less sludge to handle
- High F/M (high rate of wasting)
 - organisms are saturated with food
 - low treatment efficiency

4.Floc Loading:

Give an indication about BOD concentration available to sludge microorganism at a given time

$$\text{Floc loading (kg BOD/kg MLSS)} = \frac{\text{Mass of BOD at time of mixing}}{\text{Mass of MLSS at time of mixing}}$$

Used where sludge settlement in secondary sedimentation tank is poor

FL >100 kg BOD/ kg MLSS results improvement in sludge settling

Effluent of loading rate on plant performance:

BOD loading in influent will have the following effluent

- **Effluent Quality – sludge settling properties**
- **Efficiency of plant performance**

$$1/\Theta = Yq - k_a$$

Θ : Sludge age

Y : Sludge Yield (kg/kg BOD)

Q : organic loading rate (kg BOD/kg MLSS)

Aeration tank configuration:

- **The configuration of selected Activated Sludge reactors (aeration Tank) will have effects on many aspects of plant performance and economy (running and construction costs)**

Secondary Treatment Aeration Tank "Reactor"



Batch Reactor (BR):

Activated sludge known as the fill and draw process:

-  A reactor filled with settled sewage
-  Aeration for sufficient time (generally 8-12 hours)
-  Settling the reactor contents

Cont==Batch Reactor (BR):

-  **Discharge the treated supernatant to a watercourse**
-  **Portion of the settled sludge will be wasted**
-  **The process repeated again**

Cont==Batch Reactor (BR):

**The process (BR)
Lost fervor because
of the amount of
operator control
required**



Sequencing Batch Reactor (SBR):

- **A recent modification of BR process know as a sequencing batch reactor (SBR) is gaining increased popularity.**

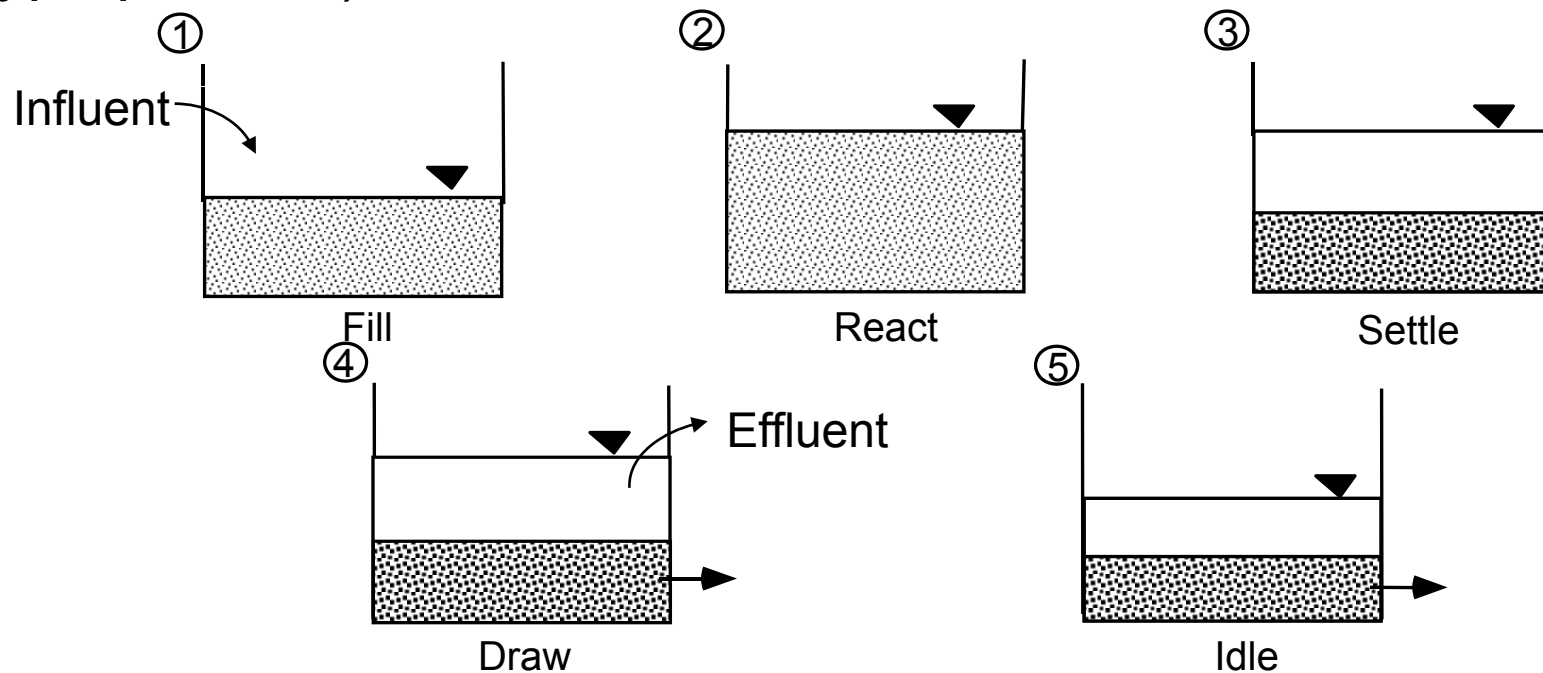
Cont==Sequencing Batch Reactor (SBR):

SBR allows different removal processes to be performed in the same reactor :

- **Carbonaceous oxidation**
- **Nitrification , Denitrification**
- **Phosphate removal**

Sequencing Batch Reactor (SBR)

- Suspended growth system
- Completely mixed mode; batch mode with discontinuous flow
- Typical $F/M = 0.05 \sim 0.1$ (comparable to an extended aeration type process)



Causes suitable to apply RBC technology:

- **Highly variable hydraulic and organic loads**
- **Small communities**
- **Vacation resorts and institutional facilities**

Complete Mix Reactors (CMR):

- 1. Rapidly distributed of settled sewage and return activated sludge**
- 2. MLSS, BOD and O₂ concentration should have the same values in any point in the reactor**

Cont==Complete Mix Reactors (CMR):

Advantage: 😄

- Large dilution of influent provides a buffer against any toxic substances
- Uniform distribution of load ensure an efficient use of aerators

Cont==Complete Mix Reactors (CMR):

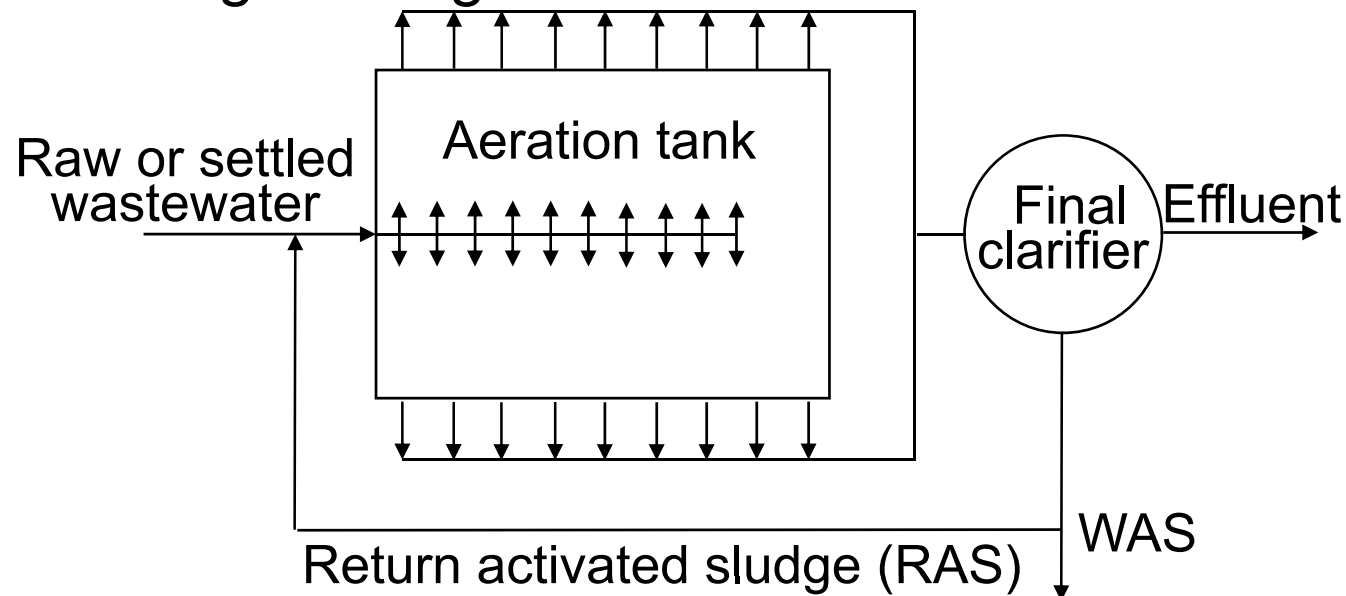
Disadvantages:



- **Short-circuiting , leads to not adequate treatment.**
- **Low floc loading , leads to sludge settlement problems.**

Completely-Mixed Activated Sludge (CMAS)

- Objective is to mix aeration basin completely.
- CMAS is inflexible (one basin); also CMAS systems tend to bulk more than conventional systems at low F/M; CMAS is useful to disperse toxic or shock loads and even out DO demand for high strength wastewater.



Oxidation Ditches:

Originally designed as:

- **Inexpensive treatment system for small rural communities**
- **Shallow (1m) continuous oval ditches**
- **Excavated directly or with simple lining for certain soil types**

Cont==Oxidation Ditches:

- **Operated at very low loading rates**
- **No need for primary sedimentation**
- **A horizontal paddle aerator provided mixing and aeration**

Cont==Oxidation Ditches:

- **Mixed liquor keep in circulation around the ditch**
- **After a suitable period of aeration , the aerator turned off and the ditch served as secondary sedimentation tank**

Cont==Oxidation Ditches:

- **Low loading rates sludge ages result:**
 - 1. Low sludge production (as result of endogenous respiration)**
 - 2. Effluent completely nitrified**

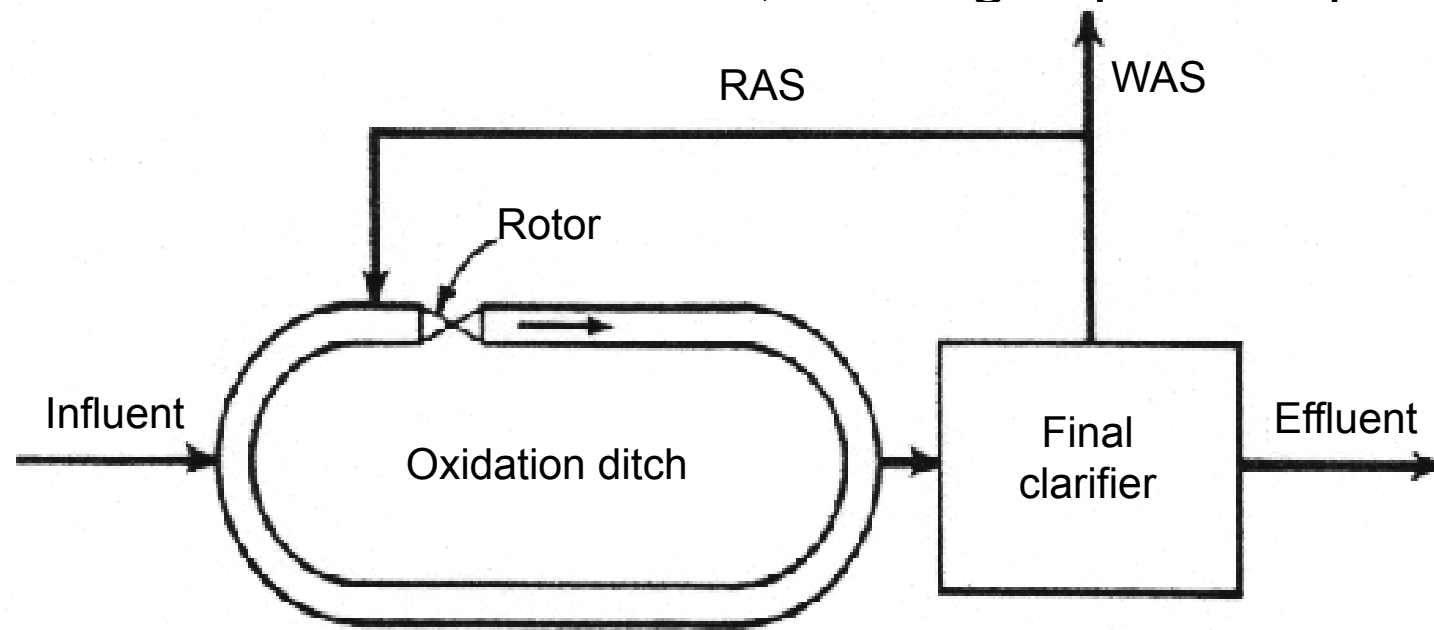
Oxidation Ditches:



Low Rate Activated Sludge

Oxidation Ditch

- Low rate, suspended growth system
- Can be operated intermittently or continuously. Continuous operation requires secondary clarifiers.
- Loading rates = 10~15 lb BOD₅/1000 ft³·day
- Useful for small communities; but large space required.



Modified oxidation ditches:

- **Represent high wastewater treatment technology**
- **Separation secondary settlement tanks**
- **The ditch is invariably constructed using concrete**

Cont==Modified oxidation ditches:

- **Vertical shaft cone aerators employed**
- **Carrousel process types for large populations**





on
Ditch



Plug-Flow Reactors:

- 1. Tanks with a high length to breadth ratio**
- 2. The reactor consists of a number of tanks or pockets in series , each equipped with its own aerator (pocket behaves a complete mix reactor)**

Cont==Plug-Flow Reactors:

- 3) Settled sewage and return sludge introduced at one end and remove at the other**
- 4) Samples along the tank show a gradient of decreasing BOD , increasing MLSS and decreasing oxygen demand**

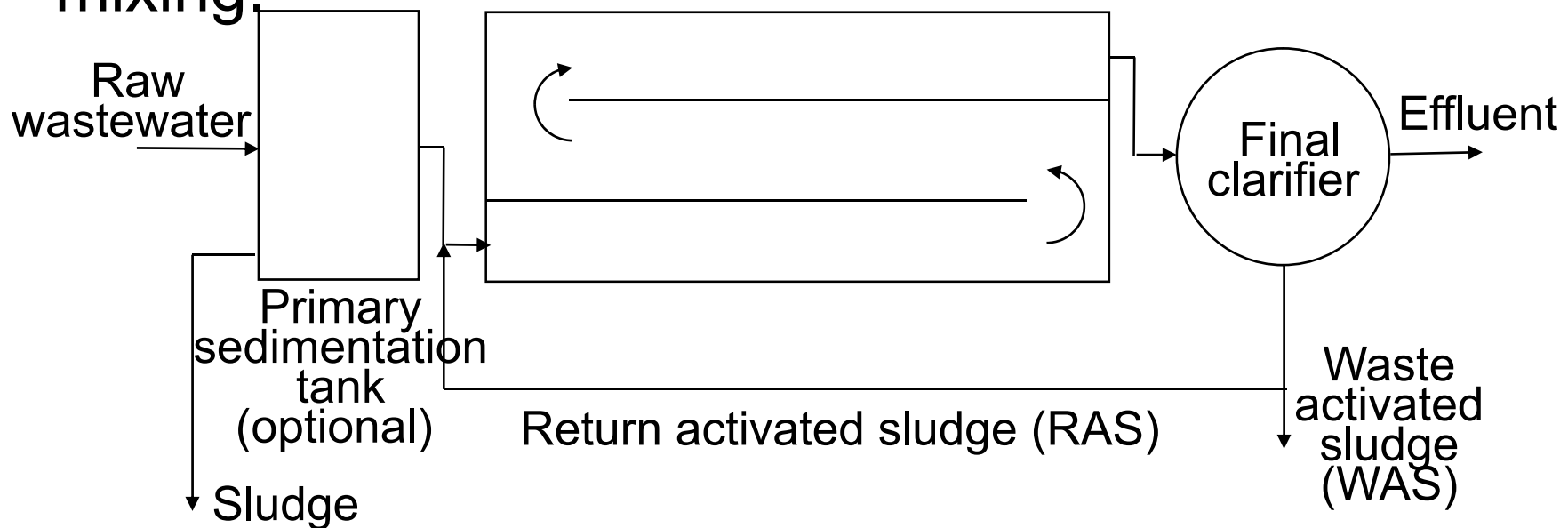
Cont==Plug-Flow Reactors:

- 5) As load and oxygen demand not evenly distributed along , step aeration is apply**
- 6) This modification of air supply matches the BOD demand and is utilized more efficiently**



Plug-Flow

- RAS and sewage are fed at one and the same point in aeration basin.
- The basin configuration will determine degree of mixing.



Cont==Plug-Flow Reactors:

- 7) Advantages : reducing total plant running costs up to 50%**
- 8) Floc loading problem of sludge settlement traditionally reduced**

Step Feeding:

- 1) **The technique is employed only on plug-Flow reactors with the aim of :**
 - **Distributing the load more evenly**
 - **Avoiding oxygen deficiency at the inlet**

Cont==Step Feeding:

- 2) All return sludge recycled to the reactor inlet**
- 3) The flow of the incoming settled sewage split and fed to a number of pockets**

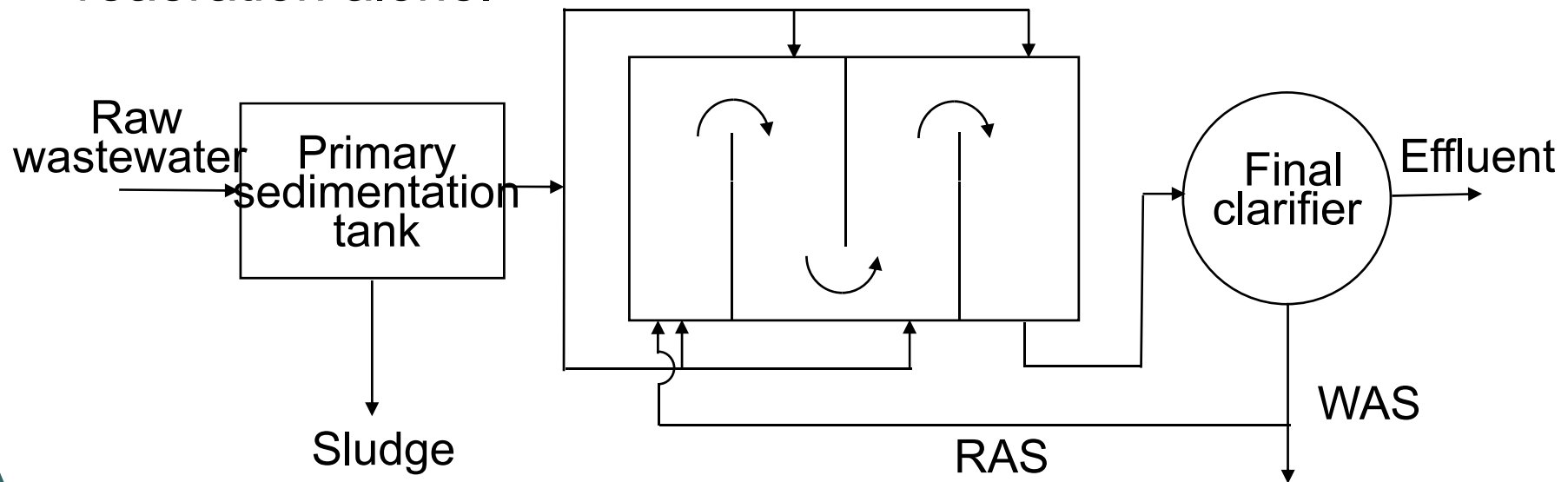
Cont==Step Feeding:

- 4) If too many increments of feed are employed , reactor will resemble in CMR



Step Aeration

- Tank size for aeration may be reduced to about the half the size of CAS.
- More efficient use of oxygen.
- Can have some part of aeration basin devoted to RAS reaeration alone.



Modifications of conventional sludge process:

Variations are possible , based on :

- **Differing reactor configuration – modes of aeration**
- **Modification has generally a specific function**

Cont== **Modifications of conventional
sludge process:**

- **Reduce construction and operation costs**
- **Modification to existing conventional plants**



Completely new plant

**{Understanding the
philosophy behind the
modification is important}**

(A/B) process:

The
absorption-bio-oxidation
(A/B) process

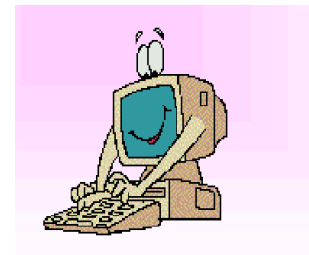


Cont==**(A/B) process:**

- **Two stage process in series**
- **The aim to maximize the production of sludge in two reactor**

Cont==**(A/B) process:**

- **Oxygen is required for oxidative process which produced energy and carbon dioxide**



Cont==**(A/B) process:**

- **The process operates without primary sedimentation**
- **A separation of the first-stage sludge from that of the second stage**

Cont== **(A/B) process:**

- **Two secondary clarifiers are required**
 - A. **The first has a very high sludge loading rate:**

Cont== **(A/B) process:**

- **3-7 kg BOD/kg MLSS d .
(operating in facultative
anaerobic mode) 50% of the
influent BOD is achieved**



Cont== **(A/B) process:**

- **Reducing energy required to remove 1 kg of BOD from 0.3 kwh to 0.15 kwh .**
- **Exploiting the gut microorganism in raw sewage.**

Cont== **(A/B) process:**

B. The B stage completes the oxidation of the sewage :

- **Loading rate 0.5kg BOD/kg MLSS d.**

Cont==(A/B) process:

- Operates aerobically with high solids retention time, with nitrification of ammonia
- Effluent ammonia concentration 5-10mg/l.

Cont== **(A/B) process:**

- **A/B process can have a loading rate 50% more than conventional single stage activated sludge plant with the same effluent quality**



Contact stabilization :

- **Process to solve the sludge settlement in CMR**
- **The process provides zone with high floc loading in a small tank**

Cont==Contact stabilization :

**Screened sewage + return
sludge 30 min in AT (contact
zone)**

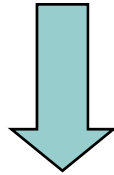
Cont== **Contact stabilization :**



Settling



effluent to wc



sludge



**transfer to stabilization zone
(aeration up to 12 hours)**

Cont== **Contact stabilization :**

- **Sludge returned to contact zone**
- **Sludge absorbed BOD to floc**

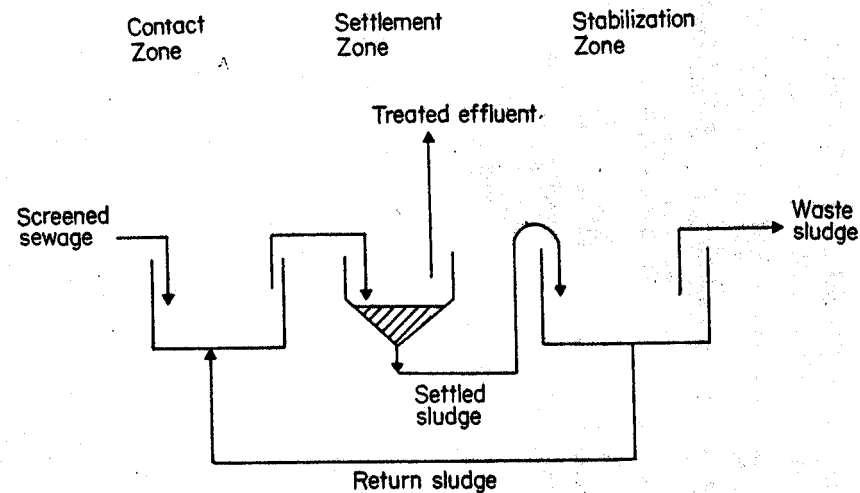
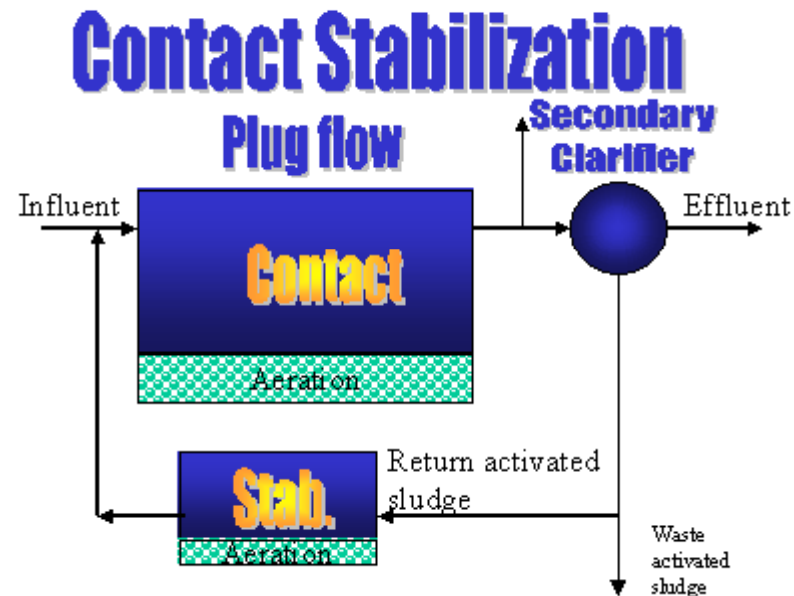


Figure 2.14 Schematic diagram of a contact stabilisation plant suitable for use in a small community.

Cont==Contact stabilization :



Cont== **Contact stabilization :**



Advantages :

**large reduction of AT
Sludge for a package plant for
small communities**

The absorption-Bio-oxidation (A/B) process:

- **The aim is to maximize sludge production**
- **Has two stage AS**
- **The process operates without primary sedimentation**

Cont== **The absorption-Bio-oxidation
(A/B) process:**

- **Two secondary clarifiers are required**
- **First stage (A-stage):**
 - **Has very high sludge loading rate (3-7kg BOD/kg MLSS/d)**
 - **Facultative anaerobic**

Cont== **The absorption-Bio-oxidation
(A/B) process:**

- **Treatment efficiency is low**
- **Rapid BOD removed (50%)**
- **Energy required low
(0.15kwh/1kg BOD)**
- **BOD removed mechanism is
absorption**

Cont== **The absorption-Bio-oxidation
(A/B) process:**

- **B-stage:**
 - **Has low sludge loading
(0.3kgBOD/kg MLSS)**
 - **Aerobic with high solids R.T**
 - **Nitrification is occur**

Cont== **The absorption-Bio-oxidation
(A/B) process:**

- **This system save in aeration cost**
- **Sludge (organic mater) used CH₄ produced (anaerobic Digestion)**



QUIZ



True or false:

- 1. The aim of primary treatment To prevent blockage of pipes**
- 2. The aim of Grit removal is to prevent organic material sedimentation**
- 3. 40% of BOD5 remove in primary sedimentation**

True or false:

- 4. **Settlement zone Should free from Short-circuiting**
- 5. **Fixed film processes represent the oldest form of wastewater treatment system**
- 6. **trickling filters Simple technique to design ,operate and high running costs**

True or false:

- 7. RBC elimination of pounding and clogging**
- 8. The average time, which a microorganism will spend in the reactor is known sludge age**
- 9. Coarse bubble diffused air Reduced oxygen transfer**

True or false:

10. **CMR rapidly distributed of settled sewage and return activated sludge**





Thanks

