

POTABLE WATER DISTRIBUTION SYSTEM MODELLING USING WaterGEMS

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"All models are approximations. Essentially, All Models Are Wrong, but some are useful. However, the approximate nature of the model must always be borne in mind."

George E. P. Box

One Of The Great Statistical Minds of The 20th Century

- A model is only as good as the data used to build it (garbage in garbage out)
- Be aware of your sources and the accuracy of your data
- Keeping your model up to date is as important as developing a model





- Historical Timeline
- Common Hydraulic Model Uses
- Common Challenges in Developing Hydraulic Models
- Water System Elements
 - Pipes
 - Valves
 - Pumps
 - Tanks



Pre 1970 - 1990s 1990s Present

Manual Calculations and physical models (too expensive)

Programming was used to simulate small system

EPANET was introduces by USEPA

Integration with GIS was presented

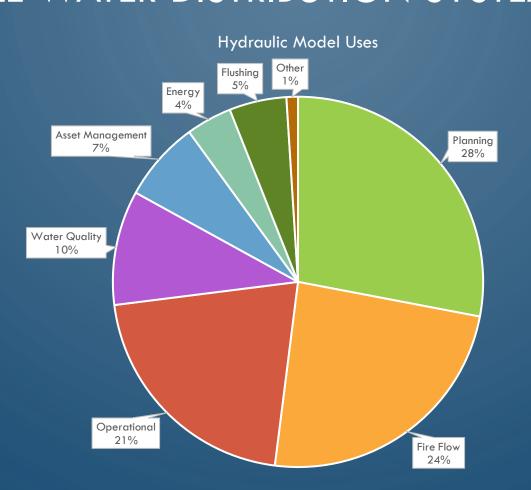
Adding water quality modelling, fire flow analysis etc.



Main purposes of hydraulic modelling:

- Planning
- Design
- Operation
- Water Quality improvement







The most technically challenging aspects of the hydraulic model (n=79)





- Water System Elements
 - Pipes
 - Pumps
 - Valves
 - Tanks



- PIPES and Fittings
 - Material
 - Polyvinyl Chloride (PVC)
 - Polyethylene (PE)
 - Asbestos Cement (AC)
 - Steel
 - Ductile Iron
 - Cast Iron

- Physical Properties
 - Diameter (Inner Diameter)
 - Roughness
- Location
 - X Coordinate
 - Y Coordinate
 - Z Coordinate (elevation)



- Pipe Flow Equations
 - Continuity Equation
 - Energy Principal
 - Hazen-Williams Formula
 - Darcy-Weisbach Formula



Flow Equations – Continuity Equation

$$Q = AV$$

Q is the flow (m³/sec)
A is the Area of flow (m²)
V = Flow Velocity (m/sec)

Flow in (Q_1)

Flow Out (Q₂)

$$A_1V_1 = A_2V_2$$



Energy Equation

$$\frac{P_1}{\gamma} + Z_1 + \frac{V_1}{2g} + H_G = \frac{P_2}{\gamma} + Z_2 + \frac{V_2}{2g} + H_L$$

Where

 $P = \text{Pressure (N/m}^2)$

 γ = Specific Weight of the fluid (Water) (N/m3)

Z = Elevation above the datum (m)

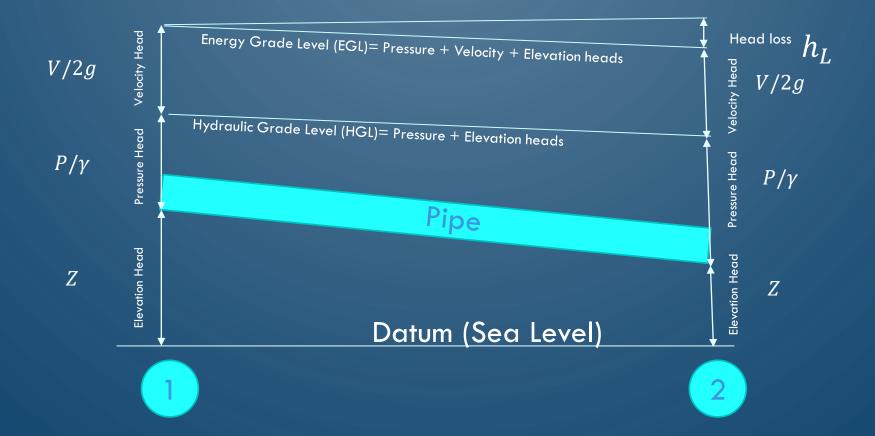
V =Water Velocity (m/s)

 $g = \text{gravitational acceleration (m/s}^2)$

 H_G = Head Gained such as from a pump (m)

 H_L = Head loss such as friction loss (m)

Energy Equation



Hazen-Williams Equation

$$V = 0.85 C R^{0.63} S^{0.54}$$

Where V = Water Velocity (m/s)

C = Hazen-Williams Roughness Coefficient (unitless)

R = Hydraulic Radius (m)

= Area (m^2) /Witted Perimeter (m)

S = Slope (m/m)





Hazen-Williams Equation – Cont.

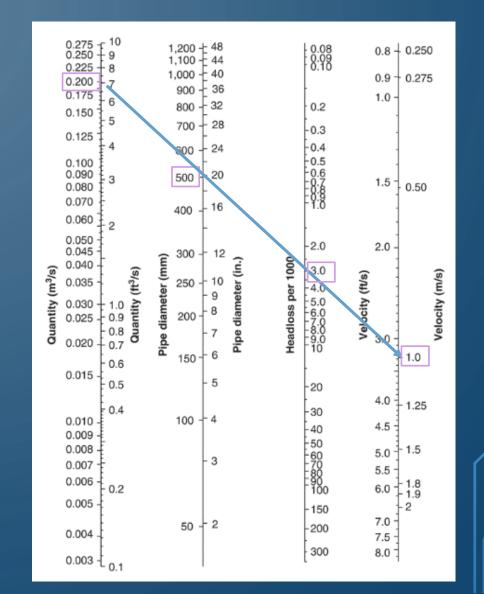
Table 5-2. Values of the Hazen-Williams coefficient C_{HW} and Manning's n for common pipe materials

Type of Pipe	c_{Hw}	n
Polyvinyl chloride (PVC) pipe	150	0.008
Very smooth pipe	140	0.011
New cast iron or welded steel	130	0.014
Wood or concrete	120	0.016
Clay or new riveted steel	120	0.017
Old cast iron or brick	100	0.020
Badly corroded cast iron or steel	80	0.035



Hazen-Williams Equation – Cont.

Nomogram for Solving the Hazen–Williams equation (C=100).



Darcy-Weisbach Equation

$$V = \sqrt{\frac{8g}{f}}RS$$

Where

V = Water Velocity (m/s)

f = Darcy-Wiesback friction factor (unitless)

R = Hydraulic Radius (m)

= Area (m^2) /Witted Perimeter (m)

S = Slope (m/m)

Darcy-Weisbach Friction Factor (unitless)

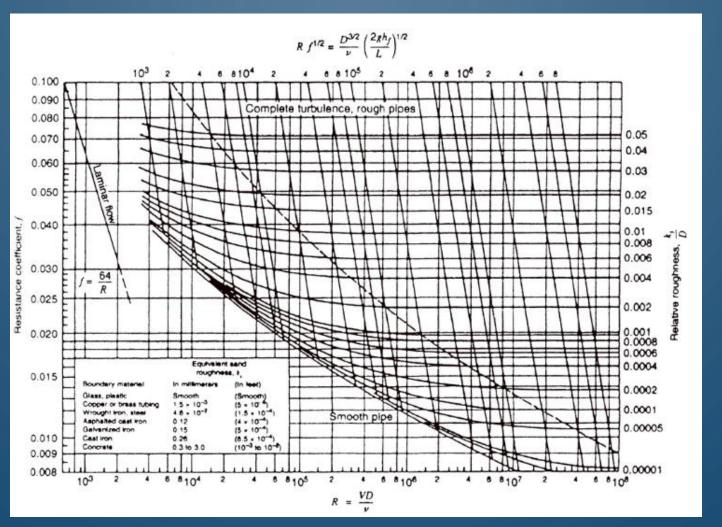
$$f = \frac{1.325}{\left[log_e\left(\frac{k}{3.7D} + \frac{5.74}{Re^{0.9}}\right)\right]^2}$$

Where f = Darcy-Wiesback friction factor

k = roughness height (m)

D = Pipe diameter (m)

Re = Reynolds number (unitless)



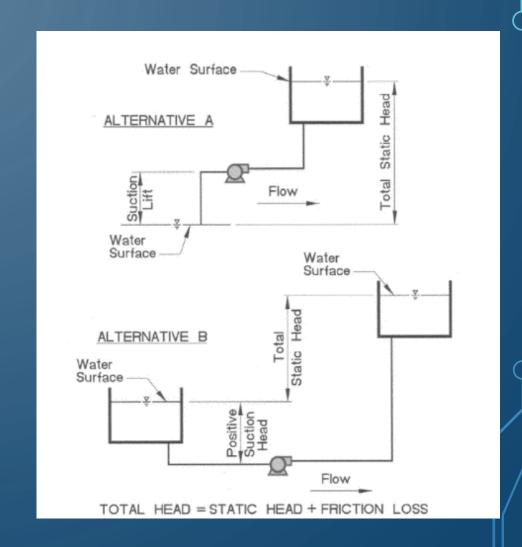
Moody diagram with the friction factor f, as a function of Reynolds number R, in Darcy–Weisbach formula for flow in conduits.



Pumps

A water pump is a device whose main job is to increase water pressure to transfer the water, or liquid, from one place to another.

- Type of water pumps
 - Centrifugal Pumps
 - Vertical Line Shaft Turbine Pumps
 - Submersible Pumps



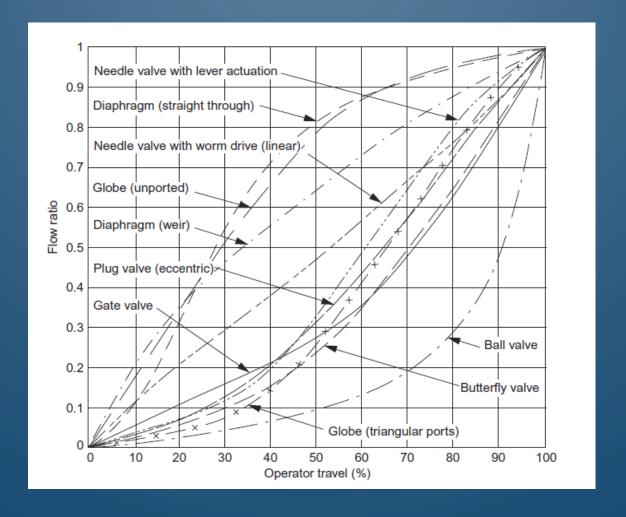


Valves

- Isolating Set either closed or fully open and normally not operated in flow conditions
- Regulating Set with any degree of opening to regulate flow and capable of periodic adjustment to opening
- Control Used with autonomous or external systems to respond to changes in flow or pressure conditions to achieve a set result which itself is capable of being reset
- Non-return Prevents reverse flow when downstream pressure is higher
- Venting To exhaust or admit air



Valves – Cont.





- Valves
 - GATE VALVES
 - BUTTERFLY
 - GLOBE VALVES
 - BALL VALVES
 - DIAPHRAGM VALVES





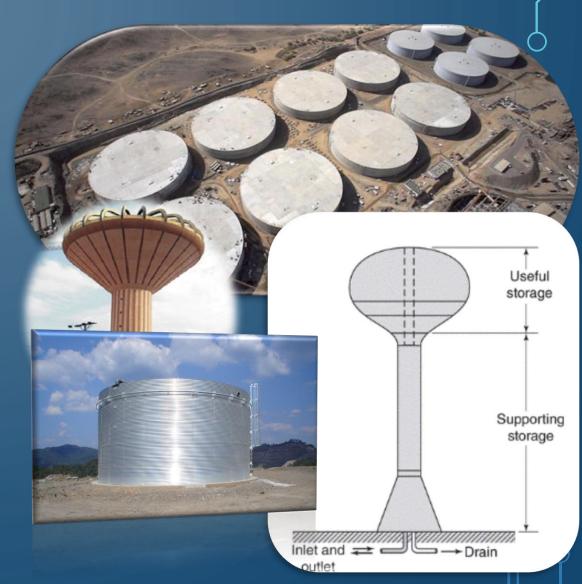
Tanks

Operational tanks

Usually integrated within the distribution network to equalize the water flow between the low consumption and the peak hours

• Strategic tanks

Usually located after the water production facility or within the transamination system





ESTABLISH THE PURPOSE AND THE NEEDS

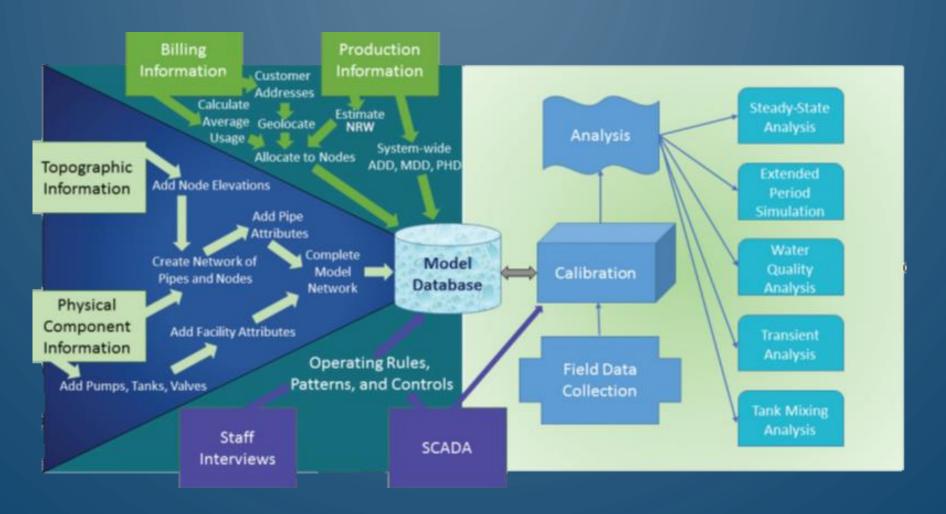
- Planning
 - Capital ImprovementProgram
 - Water MainRehabilitation Program
 - Reservoir Siting

- Design
 - Sizing the water system (mains, pump stations, PRV, tanks, etc ..)
 - Fire Flow Studies
 - Zone BoundarySelection

- Operation
 - System Trouble shouting scenarios
 - Water Loss Calculations
 - Energy Conservation scenarios



- Collect data.
- Develop the model.
- Calibrate the model.
- Analyze the distribution system.





- Data Collection Geographical data
 - Land use and zoning for existing and future conditions
 - Aerial photographs
 - Elevation data: digital terrain model (DTM), triangulated irregular network (TIN), contour maps, GPS, or other elevation data
 - Street maps and parcel maps: including street names, rights-of-way, and city and township boundaries

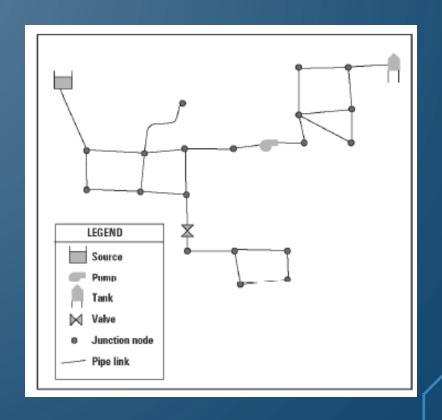


- Data Collection Existing Facilities Inventory
 - Pipes (Location, Diameter, Length, Age, Material, Lining, connectivity)
 - Storage Facilities (Location Capacity, Dimension, base water surface level, maximum water surface level, operation hours)
 - Service Connections (location, elevations, size, etc.)
 - Pumping and booster stations (elevation, number of duty pumps, spare pumps, pumps curves, sumps elevations, on-off levels, etc.)
 - Valves (type, location, diameter, etc.)
 - Other system components as needed)



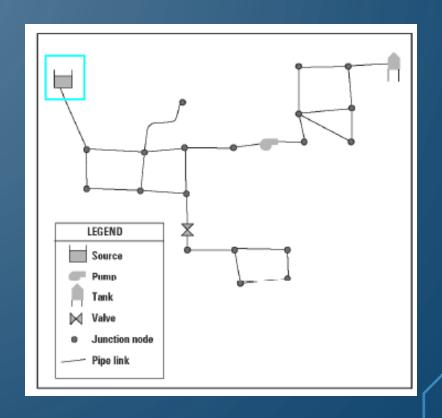
- Data Collection
 - Water meter consumption
 - Production records
 - Operating records
 - Annual, seasonal, diurnal curve
 - Physical Inspection

- Model Elements
 - Nodes (Junctions)
 - Storing Elevations
 - Diversion point (entrance and exist of water from the system)
 - Boundary condition (Valve, Pump, Tank)
 - Links
 - Represents the pipe



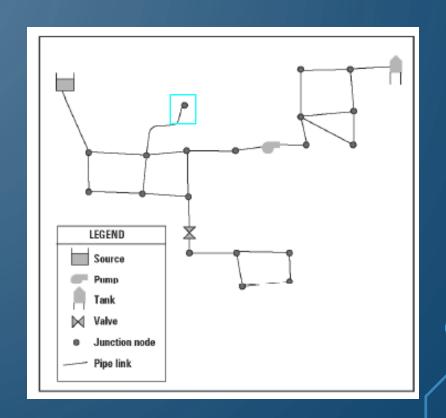
Model Elements

Model Element Type	Data Required
Reservoir	Location (N, W)
	Elevation (m)

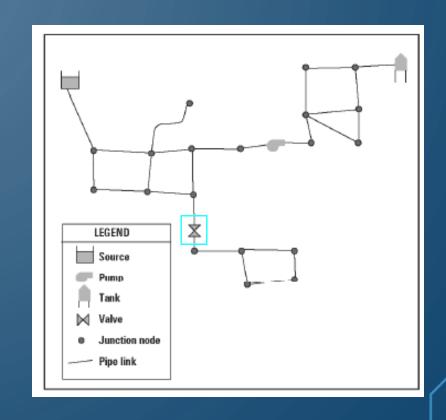


Model Elements

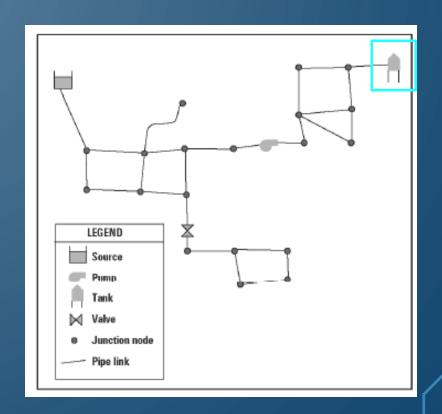
Model Element Type	Data Required
Junction	Location (N, W)
	Elevation (m)
	Demand, if exist (m ³ /day)



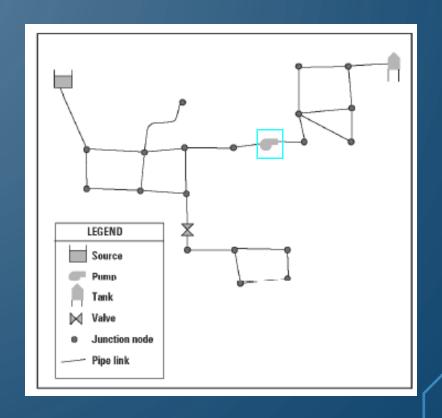
Model Element Type	Data Required
Valve	Location (N, W)
	Elevation (m)
	Status (on/off)
	Type (control, PRV, NRV, etc.)



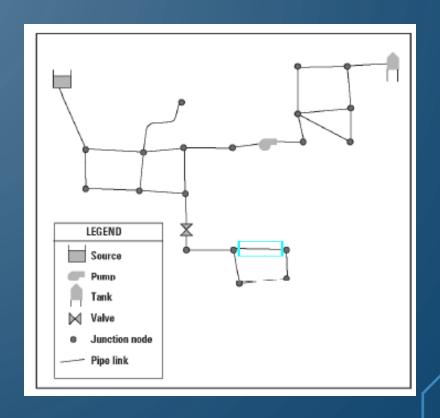
Model Element Type	Data Required
Tank	Location (N, W)
	Ground Elevation (m)
	Minimum Water Level (m)
	Maximum Water Level (m)
	Volume (m ³)
	Туре



Model Element Type	Data Required
Pump	Location (N, W)
	Elevation (m)
	Operation data
	Pump Curve



Model Element Type	Data Required
Pipe	Location, Start, End (N, W)
	Ground Elevation, Start and End (m)
	Diameter
	Material
	Head Loss coefficient (C or $\mathcal E$)

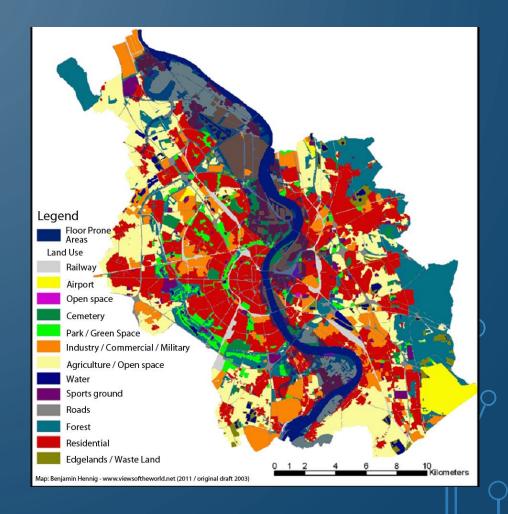




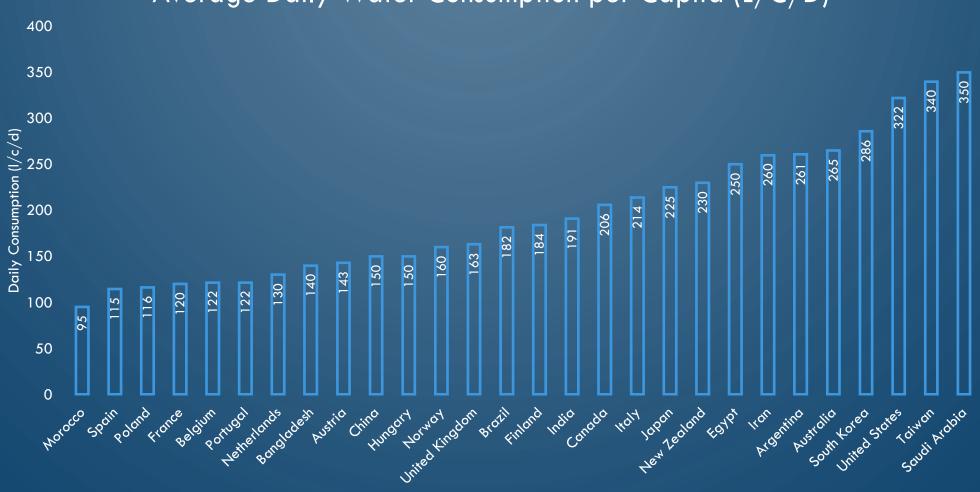
- Demand Development
 - Types of Demand (related to the land use types)
 - Residential Demands, indoor water used in flushing toilets, washing, cooking, and drinking
 - Commercial Demands, stores, restaurants, gas stations, offices.
 - Industrial Demands.

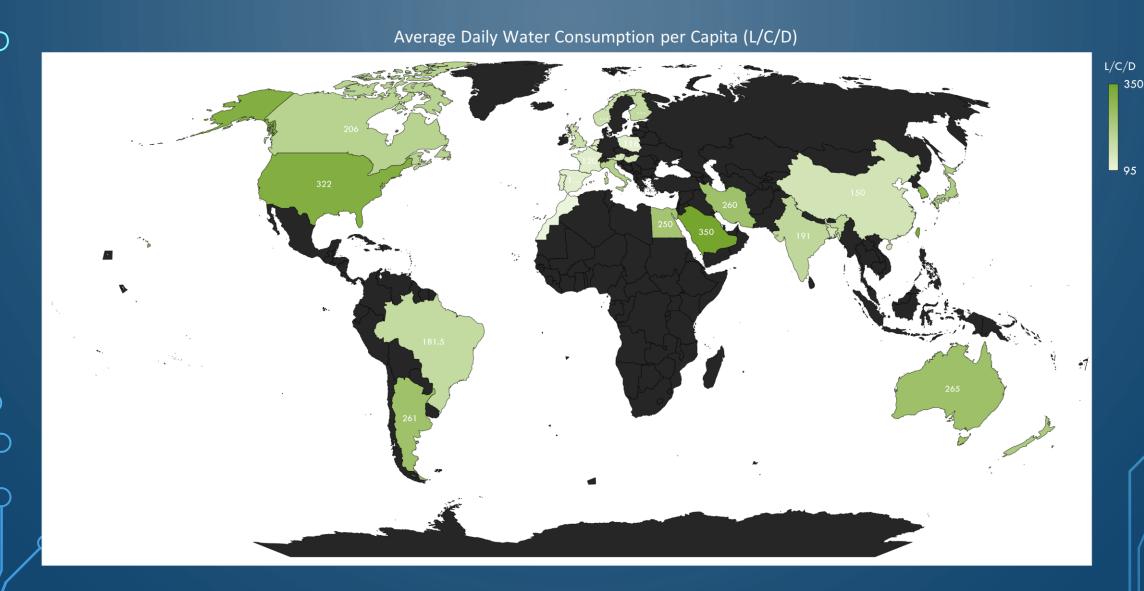


- Demand Allocation Process to nodes
 - Based on land use map using GIS methodology
 - Manual process



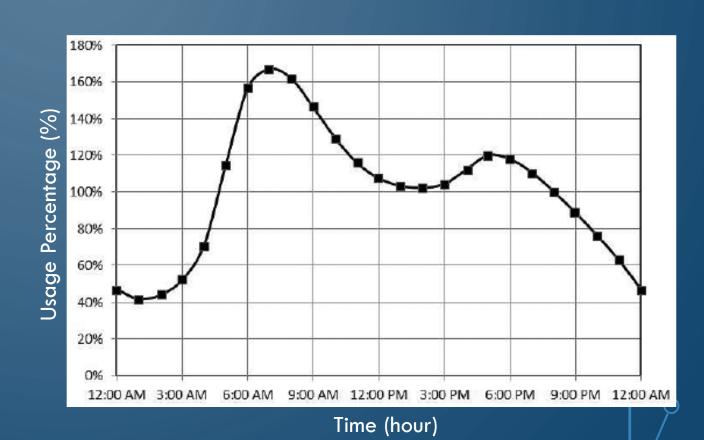
Average Daily Water Consumption per Capita (L/C/D)







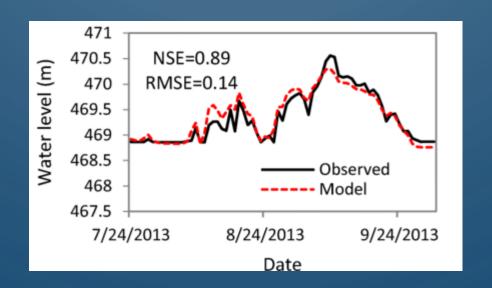
- Temporal Variation
 - Seasonal
 - Seasonal Tourism (Hajj and Omrah)
 - Daily
 - User's behaviour (Residential)
 - Land use related variation (Offices)







Calibration is <u>comparing</u> between the developed model to the actual field data and <u>making corrections and adjustments</u> to the model to achieve close agreement between computer-predicted values and field measurements.





Calibration Goals:

- Hydraulic grade line (HGL)
 - Predicted values between \pm 2.2 and 4.3 PSI of the recorded in the field
- Water Reservoir Top Water Levels
 - Predicted water levels between \pm 0.9 and 1.8 m

Model Uses:

- Planning
- Pipeline sizing
- Fire flow analysis



Calibration Goals (As required by the AHJ):

- Hydraulic grade line HGL
 - Predicted values between ± 2.2 and 4.3 PSI of the recorded in the field
- Water Reservoir Top Water Levels
 - Predicted water levels between \pm 0.9 and 1.8 m
- Water Flows (UK WRc):
 - 5% of recorded flow > 10% of the total flow demand
 - 10% of recorded flow < 10% of the total flow
- Pressure (UK WRc):
 - Difference between Model and recorded measurements:
 - (5% or 0.5 m) of recorded head loss of 85% of the test measurements
 - (7.5% 0.75 m) of recorded head loss of 95% of the test measurements
 - (15% or 2.0 m) of recorded head loss of 100% of the test measurements



Data Sources and Errors:

- System maps
- Elevation Data
- SCADA system
- Pressure Recording Devices (may not be calibrated)
- Flow Recording Devices (may not be calibrated)
- Pump Curve

- Control Valve
- Non-Revenue Water (NRW)
- Unmetered flow
- Peak Factors
- Roughness Coefficients (C-factor)
- Connectivity





STEADY STATE SIMULATION

Steady State Simulation represents a snapshot of the system under one set of condition such as:

- Average Day Demand
- Max Day Demand
- Max Hour Demand
- Fire flow Condition

>>> STEADY STATE SIMULATION

Purpose of the analysis	Scenario
Normal Operation Studies	Average Day, Maximum Hour
Production and pumping requirements	Maximum day
Sizing of pipelines—small systems	Maximum day plus fire flow
Sizing of pipelines—large systems	Maximum hour and maximum day
System reliability during emergency or planned shutdown	Condition when the emergency or shutdown is likely to occur
Model calibration	Condition during time when measurements were collected



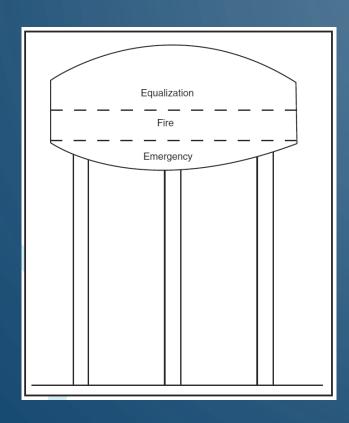


Extended-period simulation (EPS) is a technique for modeling a distribution system where a <u>series of steady-state</u> simulations at <u>specified intervals</u> are performed over a <u>specified time period</u>.

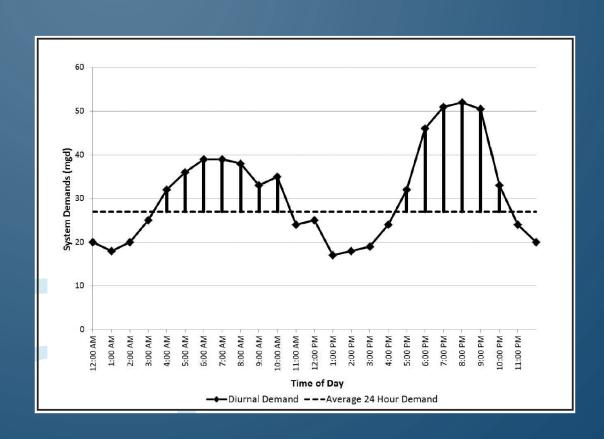


- Required Information:
 - Time interval
 - Diurnal Demand Variations
 - Tanks constrains (Water high level, Water low level, fire storage, etc..)
 - Pumps operation data and conditions (on and off controls)



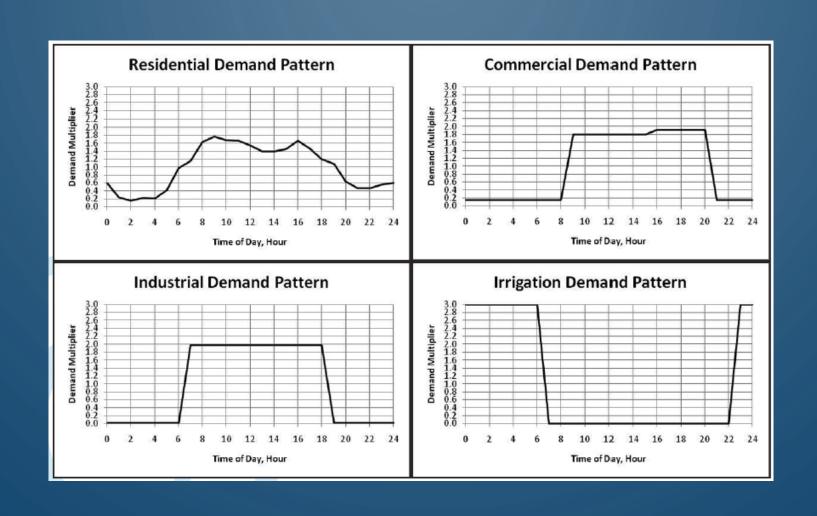


Storage Allocation

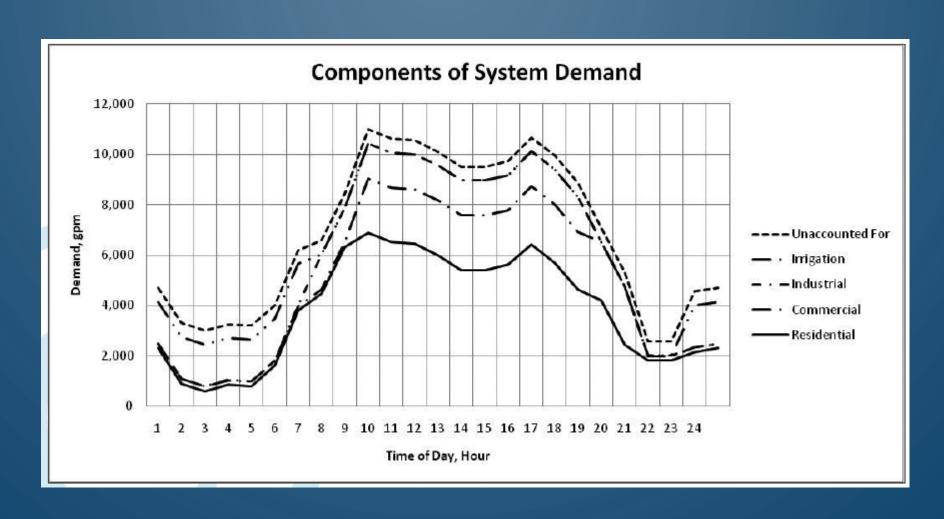


Equalization storage requirements for maximum day conditions









Purpose of the analysis	Scenario
Sizing of storage facilities	Maximum hour
Tank filling capabilities	Minimum hour of average day
System reliability during emergency or planned shutdown	Condition when the emergency or shutdown is likely to occur
Model calibration	Condition during time when measurements were collected

LIVE DEMO USING WaterGEMS

REFERENCES

- Clark, Robert M.. (2012). Modeling Water Quality in Distribution Systems (2nd Edition) 5.2.1 Friction Head Loss Formulas. American Water Works Association (AWWA).
- American Water Works Association (AWWA). (2017). Computer Modeling of Water Distribution Systems - Manual of Water Supply Practices, M32 (4th Edition) - Title Page. American Water Works Association (AWWA).
- Shammas, Nazih K. Wang, Lawrence K.. (2016). Water Engineering Hydraulics,
 Distribution and Treatment (1st Edition) 8.2 Pump Characteristics. John Wiley & Sons.
- Malcolm J. Brandt, K. Michael Johnson, Andrew J. Elphinston, Don D. Ratnayaka,
 (2017). Twort's Water Supply (Seventh Edition), Butterworth-Heinemann.

