## Lecture 12

Introduction to Environmental Engineering

# Effect of Organic Wastes on Stream Ecosystems

#### Streeter-Phelps Model – DO sag curve

- Many equations and computer programs are available today to describe the quality of water in streams, rivers and lakes
- The most prevalent is the Streeter Phelps equation.
- Addition of wastewater (BOD) typically causes a slow decrease in O<sub>2</sub>, followed by a gradual increase close to the dissolved oxygen (D.O.) saturation concentration

- Assumptions of the Model
  - stream is an ideal plug flow reactor
  - steady-state flow and BOD and DO reaction conditions
  - The only reactions of interest are BOD exertion and transfer of oxygen from air to water across air-water interface

 Mass Balance for the Model
 Not a Steady-state situation rate O<sub>2</sub> accum. = rate O<sub>2</sub> in – rate O<sub>2</sub> out + prod. – cons. rate O<sub>2</sub> accum. = rate O<sub>2</sub> in – 0 + 0 – rate O<sub>2</sub> cons.

• Both reoxygenation and deoxygenation are  $1^{st}$  order rate of deoxygenation =  $-k_1C$ 

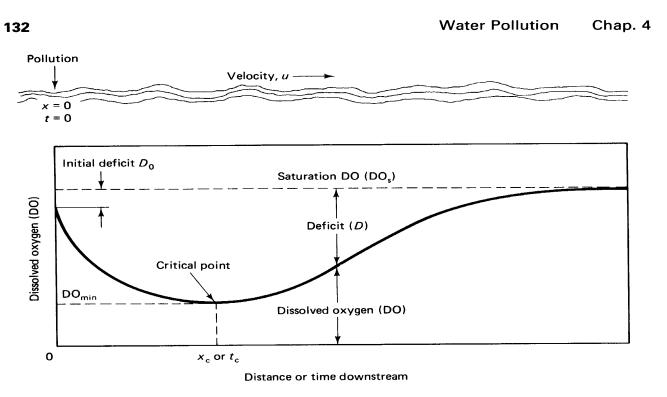
k<sub>1</sub>= deoxygenation constant, function of waste type and temperature

rate of reoxygenation =  $k_2D$ D = deficit in D.O. or difference between saturation and current D.O.

 $k_2 = reoxygenation constant$ 

$$k_{2} = \frac{3.9v^{\frac{1}{2}} ([1.025]^{(T-20)})^{\frac{1}{2}}}{H^{\frac{3}{2}}}$$

■ Where  $\bullet$  T = temperature of water, °C  $\bullet$  H= average depth of flow, m  $\bullet$  v = mean stream velocity, m/s Oxygen Deficit  $\bullet D = S - C$ D.O. deficit = saturation D.O. - D.O. in the water





for the critical time yields

$$t_{\rm c} = \frac{1}{k_{\rm r} - k_{\rm d}} \ln \left\{ \frac{k_{\rm r}}{k_{\rm d}} \left[ 1 - \frac{D_0(k_{\rm r} - k_{\rm d})}{k_{\rm d}L_0} \right] \right\}$$
(4.28)

The maximum deficit can then be found by substituting the value obtained for the critical time  $t_{into}$  (4.24)

Deoxygenation rate is equivalent to BOD of waste

 $\bullet \mathbf{r}_{o} = \mathbf{k}_{1} \mathbf{L}_{t}$  $\bullet \mathbf{L}_{t} = \mathbf{L}_{o} \mathbf{e}^{-\mathbf{K}t}$ 

 $L_o$  or L = ultimate BOD of the wastewater and stream water mixture

#### In terms of the deficit with time

$$\frac{dD}{dt} = k_1 z - k_2 D$$
$$z = Le^{-k_1 t}$$

Substiting and integrating yields the following equations

$$D = \frac{k_1 L_o}{k_2 - k_1} (e^{-k_1 t} - e^{-k_2 t}) + D_o e^{-k_2 t}$$
$$\frac{dD}{dt} = k_1 L_o e^{-k_1 t} - k_2 D = 0$$
$$D_c = \frac{k_1}{k_2} L_o e^{-k_1 t}$$
$$t_c = \frac{1}{k_2 - k_1} \ln \left\{ \frac{k_2}{k_1} \left[ 1 - \frac{D_o (k_2 - k_1)}{k_1 L_o} \right] \right\}$$

#### Example:

◆ Wastewater mixes with a river resulting in a BOD = 10.9 mg/L, DO = 7.6 mg/L
The mixture has a temp. = 20 °C
Deoxygenation const.= 0.2 day<sup>-1</sup>
Average flow = 0.3 m/s, Average depth = 3.0 m
DO saturated = 9.1 mg/L

- Find the time and distance downstream at which the oxygen deficit is a maximum?
- Find the minimum value of DO?

Initial Deficit
 D<sub>o</sub> = 9.1 - 7.6 = 1.5 mg/L
 Estimate the reaeration constant

$$k_{2} = \frac{3.9v^{1/2}}{H^{3/2}} = \frac{3.9(0.3m/s)^{1/2} \left( \left[ 1.025 \right]^{(20-20)} \right)^{1/2}}{(3.0m)^{3/2}} = 0.41 \,\mathrm{day}^{-1}$$

Calculate the time at which the maximum deficit is reached, with t<sub>c</sub>:

$$t_{c} = \frac{1}{k_{2} - k_{1}} \ln \left\{ \frac{k_{2}}{k_{1}} \left[ 1 - \frac{DO_{o}(k_{2} - k_{1})}{k_{1}L_{o}} \right] \right\}$$
$$= \frac{1}{(0.41 - 0.2)} \ln \left\{ \frac{0.41}{0.2} \left[ 1 - \frac{1.5(0.41 - 0.2)}{0.2 \times 10.9} \right] \right\}$$
$$= 2.67 days$$
$$x_{c} = vt_{c} = 0.3m / s \times 86,400s / day \times 2.67 days = 69,300m$$

The maximum DO deficit is:

$$D_c = \frac{k_1}{k_2} L_o e^{-k_1 t}$$

$$= \frac{0.2}{0.41} (10.9 \text{ mg/L}) e^{-(0.2 \text{day}^{-1})(2.67 \text{days})}$$
$$= 3.1 \text{ mg/L}$$